

AASHTO T-3 TRIAL DESIGN BRIDGE DESCRIPTION

State: Oregon

Trial Design Designation: OR-1

Bridge Name: Jackson School Road Over Hwy 47

Superstructure Type: Continuous prestressed precast Bulb" T" with composite concrete deck

Span Length(s): Two spans @ 139.44ft each

Substructure Type: Three 4.5ft. by 10.0ft. reinforced concrete columns per bent

Foundation: Steel pipe piles (closed end) 16in. dia. by 0.5in. thickness

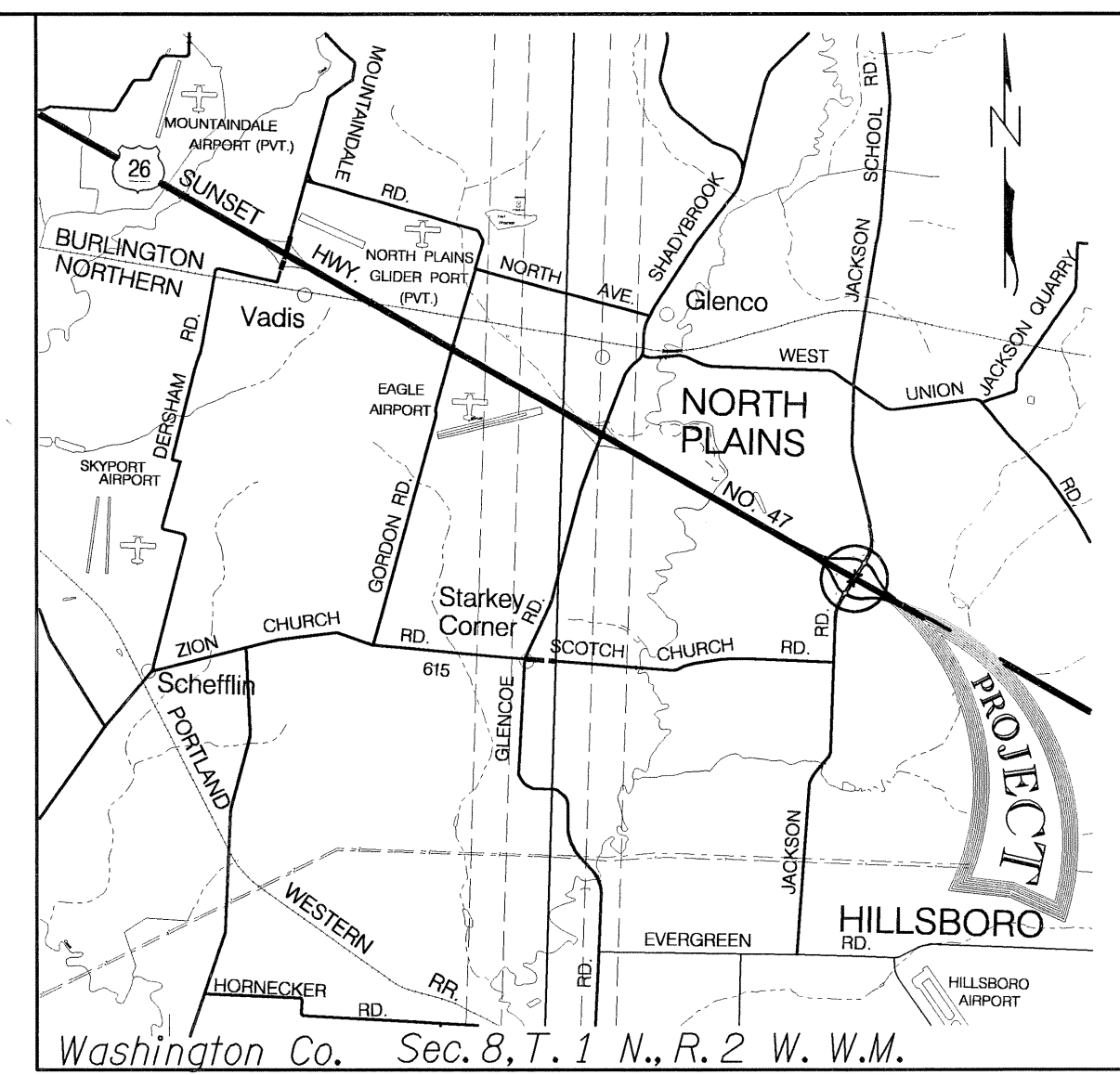
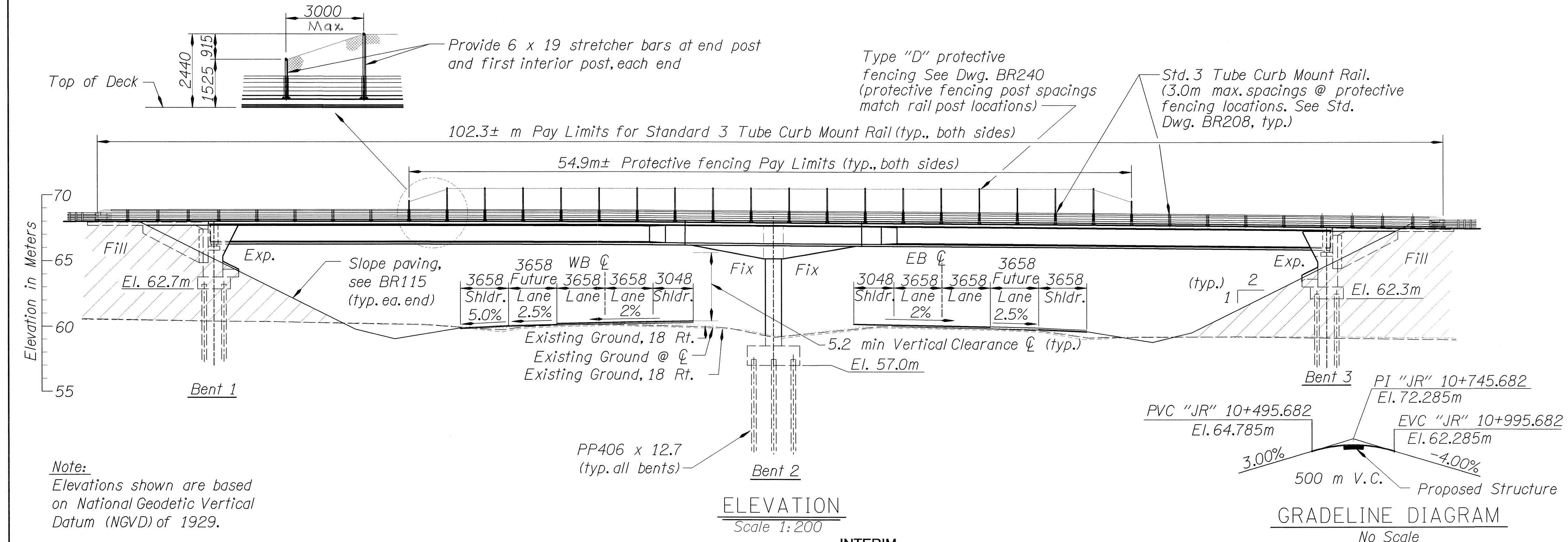
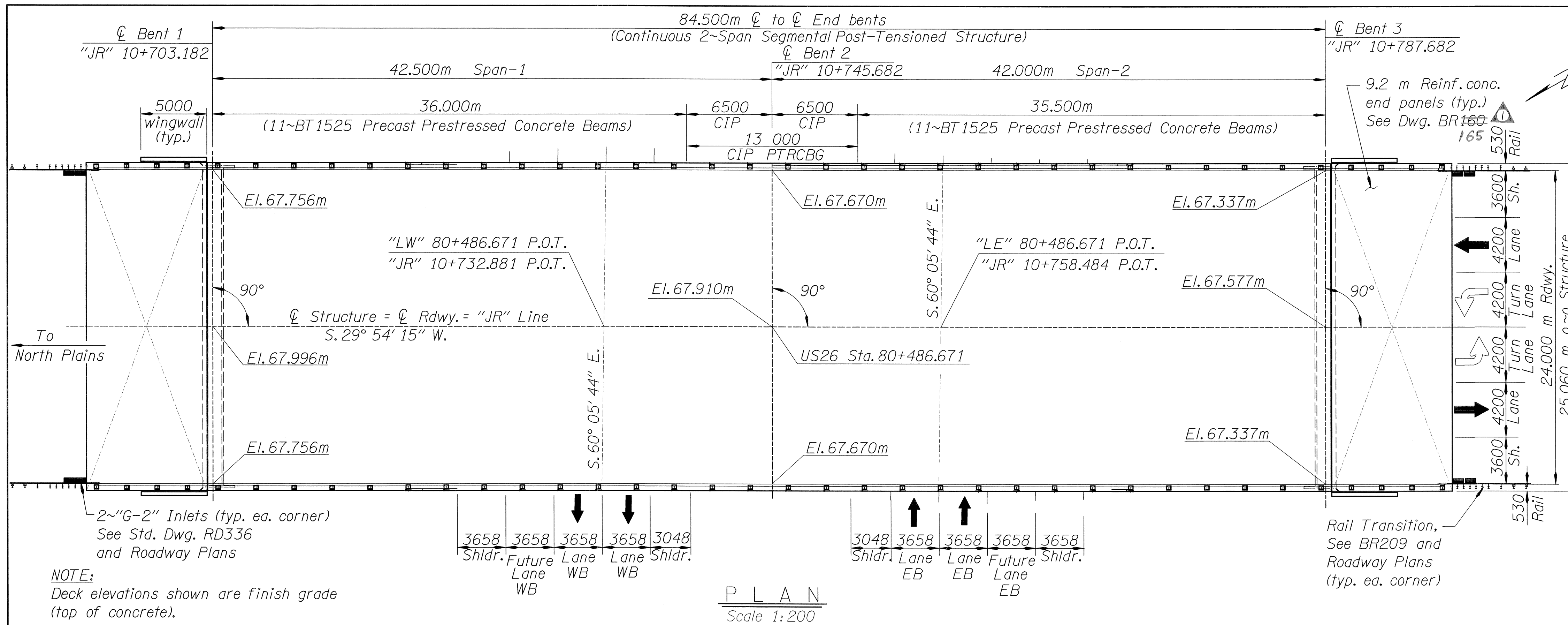
Abutments: Seat type end bents on 16in. dia. pipe piles

Seismic Design Category (SDC): _____

Seismic Design Strategy (Type 1, 2 or 3): _____

Design Spectral Acceleration at 1-second Period (S_{D1}): _____

Additional Description (Optional): Trial design is currently being done for this bridge.

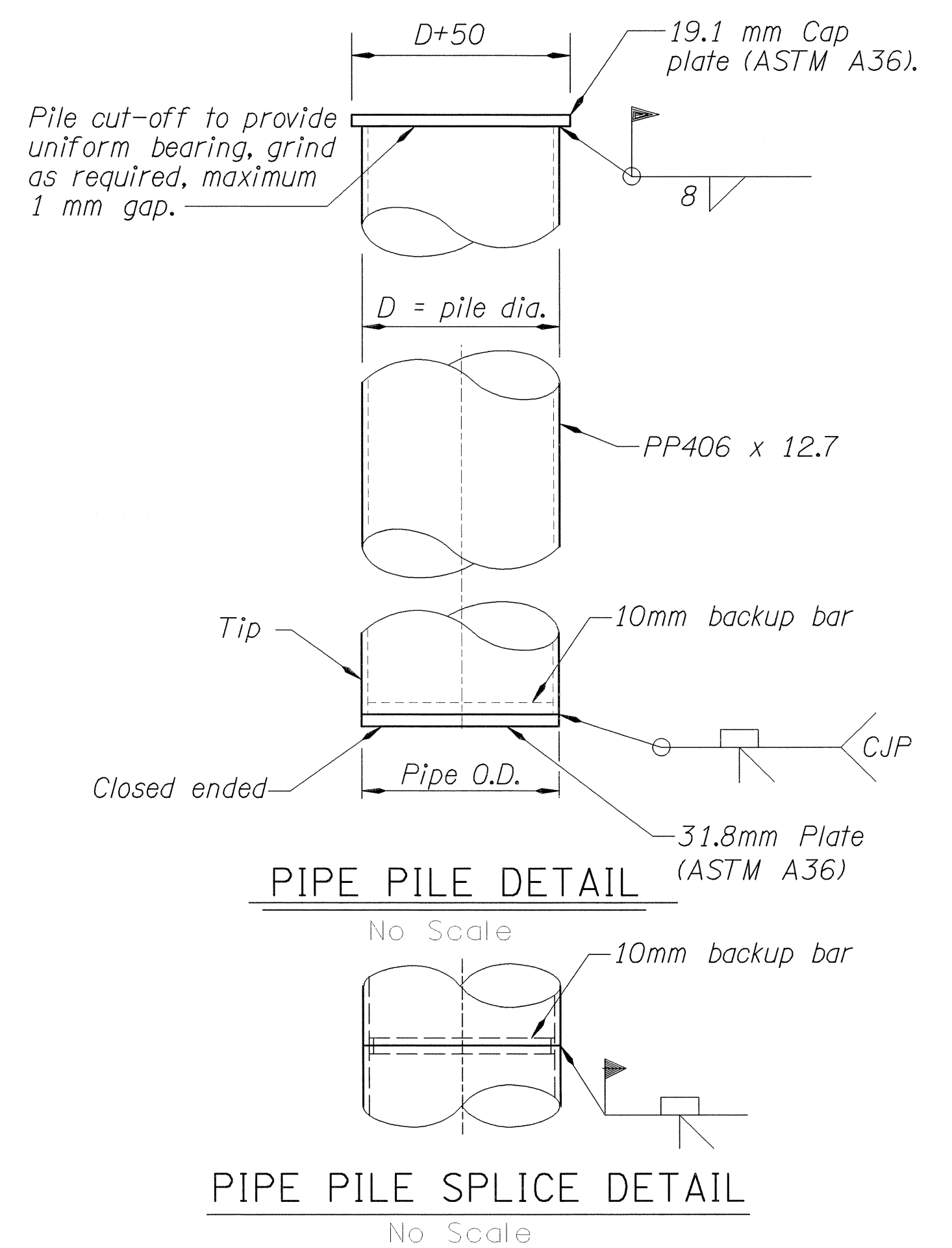
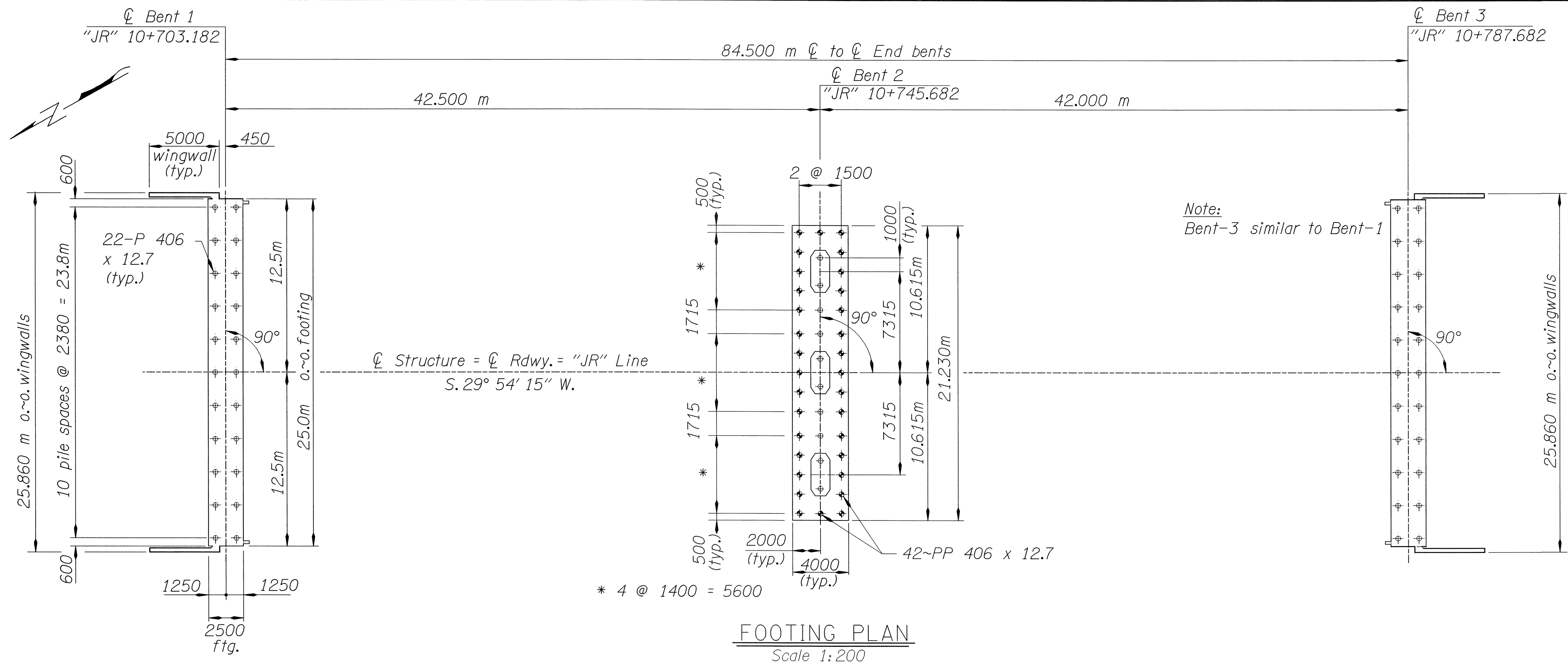


GENERAL NOTES:
 Provide all materials and perform all work according to the 2002 Oregon Standard Specifications for Construction of the Oregon Department of Transportation and the Special Provisions.
 Bridge is designed for HL-93 live load with an allowance of 1.2 kN/m² for future wearing surface.
 Bridge is also designed to meet the following permit load with future wearing surface included,
 ODOT Permit 4. Strength II Limit State- See Dwg. 65083

Concrete members are designed by the Load and Resistance Factor Design method in accordance with the 1998 AASHTO LRFD Bridge Design Specifications including 1999 to 2003 interim revisions.
 Seismic design is by multi-mode analysis in accordance with the 1998 AASHTO LRFD Bridge Design Specifications including 1999 to 2003 interim Revisions. The site peak bedrock acceleration coefficient (A) is 0.20g. The assumed soil profile is Type II. The design return period is 500 years. The Response Modification Factors used are: R=5 for column moments, R=0.8 for abutment connections and R=1.0 for other components.
 Provide spiral column reinforcement according to ASTM Specification A706M, AASHTO Specification M31 (ASTM A615M) Grade 420, AASHTO M225M (ASTM A496), or AASHTO M32M (ASTM A82).
 Provide reinforcing steel conforming to ASTM specification A706M, or AASHTO M31 (ASTM A615M) Grade 420. Where reinforcing steel is field bent, provide bars conforming to ASTM A706M. Use the following splice lengths (unless otherwise shown):

Splice reinforcing steel at alternate bars, staggered at least one splice length or as far as possible, unless shown otherwise.
 For additional general notes, see Dwg. 65077.

DATE	5-04	REVISION	Corrected drawing number	BY	EL	DESIGNER	BRIDGE ENGINEER	BRIDGE NO.	19592	DATE	19-Mar-2004	CALC. BOOK	5210-5211	FEDERAL HIGHWAY ADMINISTRATION	REGION 10	OREGON DIVISION	PROJECT NUMBER	SHEET	1 OF 20	DRAWING NO.	65076
	DRAFTED:	E. Leon	CHECKED:	Thiet Nguyen	REVIEWED:																



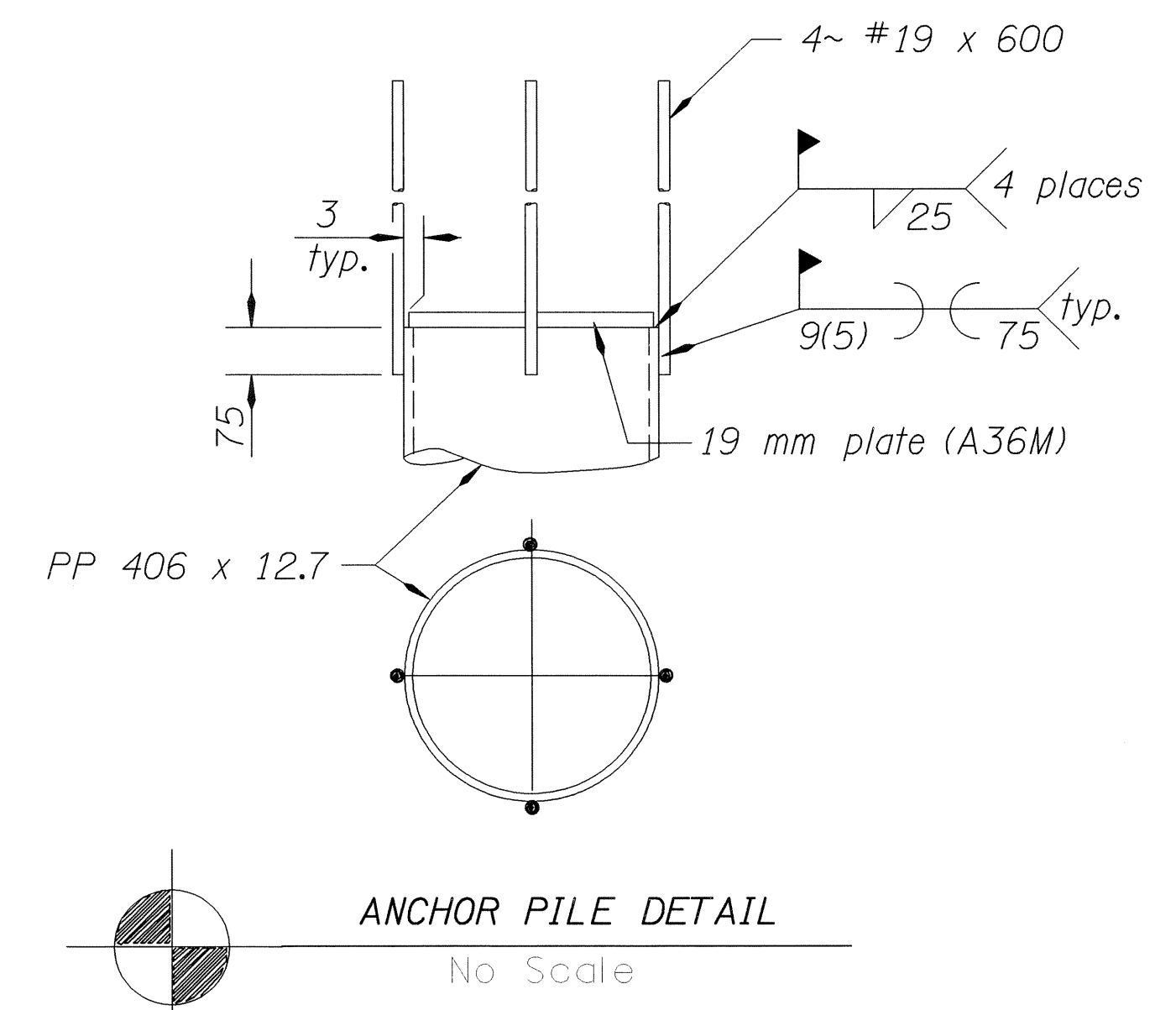
GENERAL NOTES CONTINUED:

- Support the bottom mat of reinforcing steel from forms with precast mortar blocks at 600mm maximum centers each way. Support the top mat of reinforcing steel from the bottom mat of reinforcing steel with reinforcing bar supports by Dayton Superior Co. (SBU, BBU or CHCU) or approved equal at 600mm maximum centers.
- Place bars 50mm clear of the nearest face of concrete unless shown otherwise. Field bend the top of stirrups extending from prestressed precast units.
- Provide fully threaded rods at exterior girder diaphragms according to ASTM A36M.
- Do not fabricate reinforcing steel for columns until final footing elevations have been determined in the field.
- Provide Class 50-19 concrete in prestressed precast Bulb-T beams according to the detail plans, see Dwg. 65091. Provide a minimum concrete strength of 35 MPa at transfer of prestress.
- Provide Class 35-19.0 concrete in post-tensioned box girder superstructure.
- Provide Class HPC30-19.0 microsilica concrete for the deck and closure diaphragms (Beam C).
- Provide Class 30-19.0 concrete in columns and reinforced concrete end panels.
- Provide Class 25-37.5 or 19.0 concrete at all other locations.

- Provide prestressing steel according to the detail plans.
 - Provide structural steel conforming to ASTM specification A36 unless shown otherwise. Structural steel plates are fractional inch thickness expressed in equivalent metric units.
 - For structural steel connections, provide high-strength fasteners conforming to AASHTO Specification M164 (ASTM A325, unless shown otherwise).
 - Perform all welding in accordance with the latest edition of the AWS Bridge Welding Code.
 - Hot-dip galvanize all bolts, washers, nuts, and structural steel after fabrication. Repair damaged galvanizing according to ASTM Specification A780.
 - Provide fully threaded anchor rods for resin bonded anchors according to AASHTO Specification M314, Grade 105. Select a high strength resin from the Department's Qualified Products List.
- Piling Notes:**
 Provide PP406 x 12.7 conforming to ASTM Specification A252, Grade 2 at all bents.

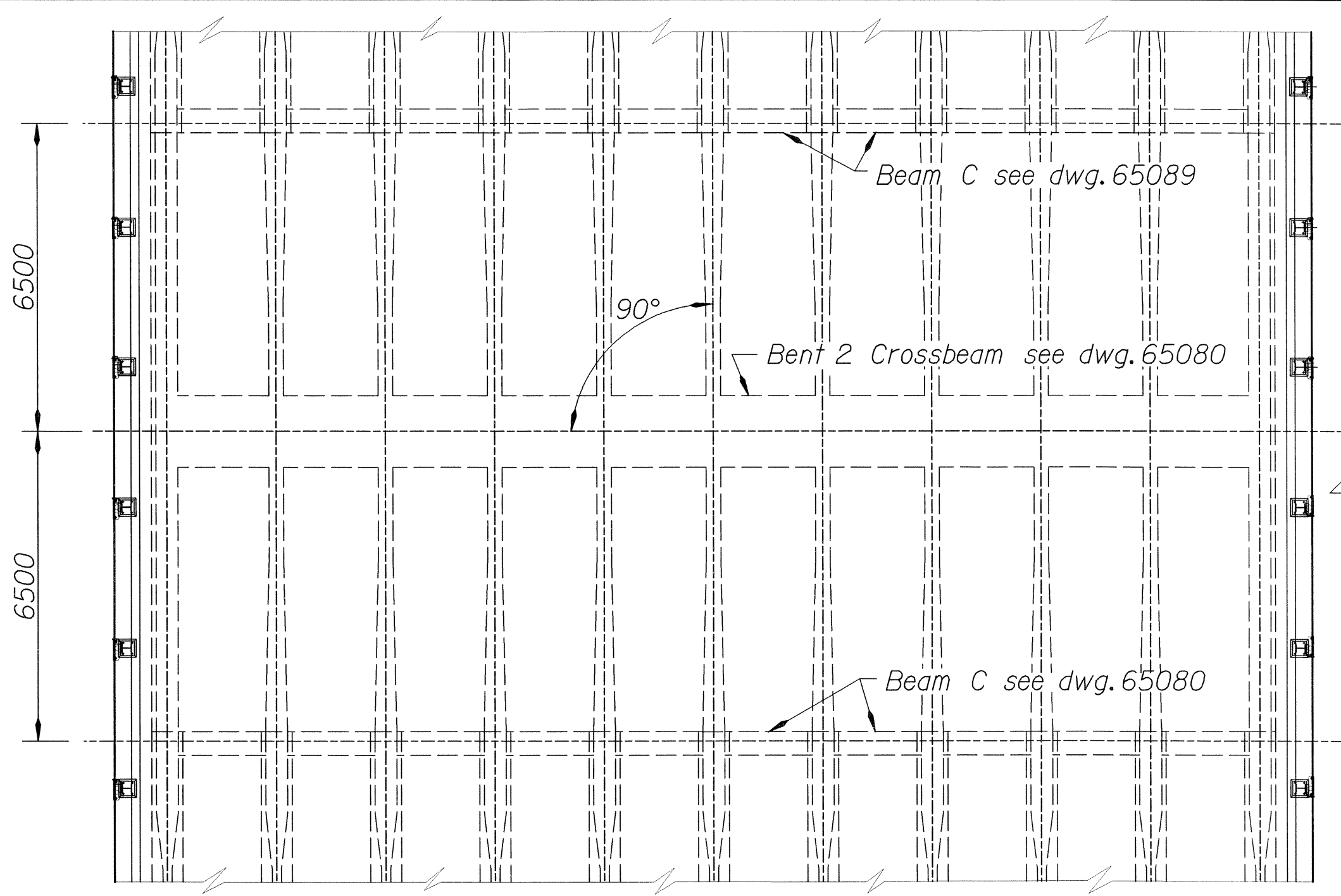
- Drive all piles closed ended to an ultimate capacity of 2315 kN using driving criteria developed from a Wave Equation Analysis.
- Pile tip elevation for minimum penetration is as follows:
 Bent-1 - Elev. 35.000 m
 Bent-2 - Elev. 33.000 m
 Bent-3 - Elev. 34.000 m

See Dwg. 63886/GHB-1 for Foundation Data

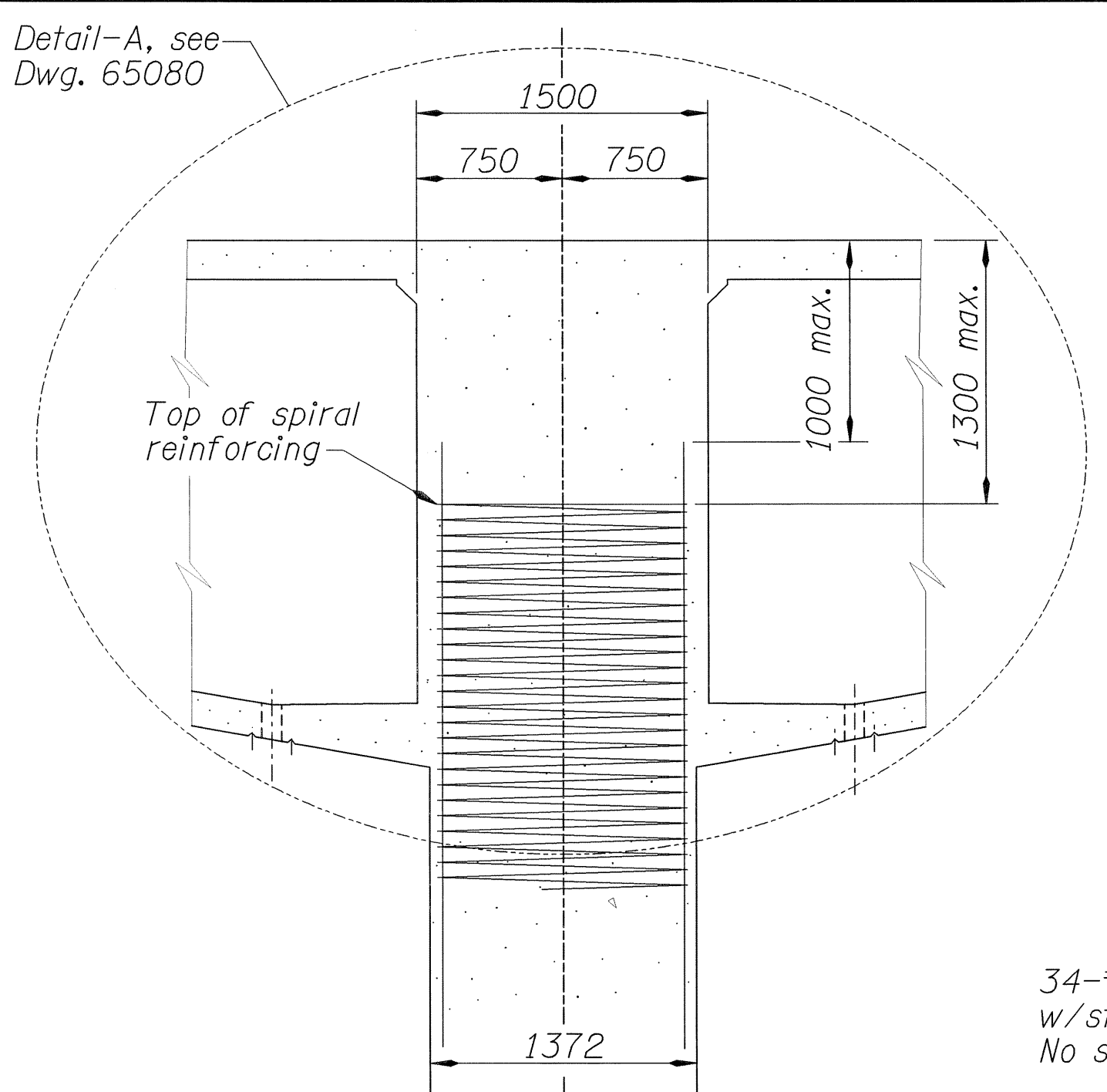


NOTE: All dimensions are in millimeters (mm) except as noted.

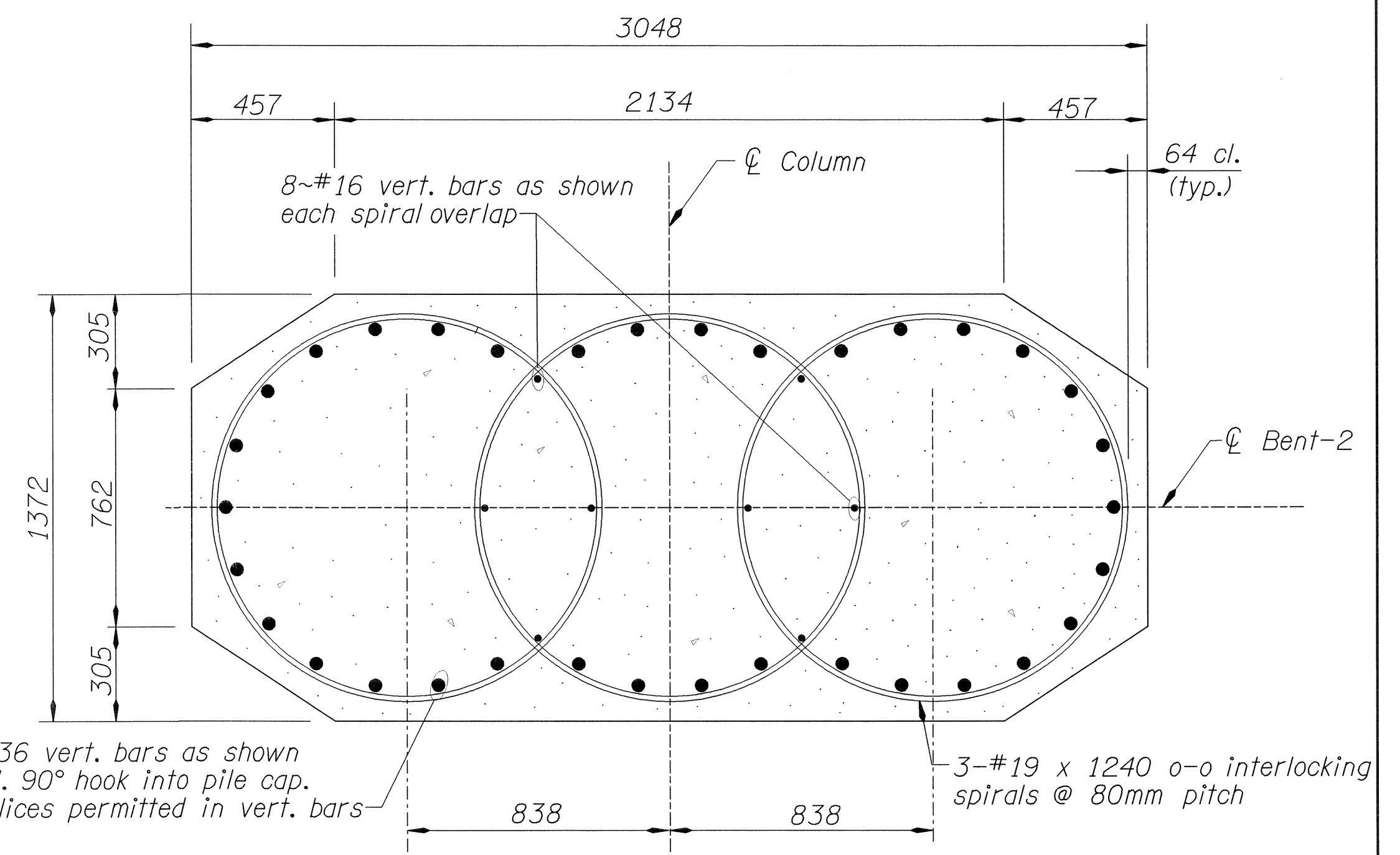
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				DRAFTED: Philip G. Amaya CHECKED: Thiet Nguyen REVIEWED: Hormoz Seradj		19592		2	
				EXPIRES: 6-30-04				DATE	OF
								19-Mar-2004	20
						CALC. BOOK	DRAWING NO.		
						5210-5211	65077		



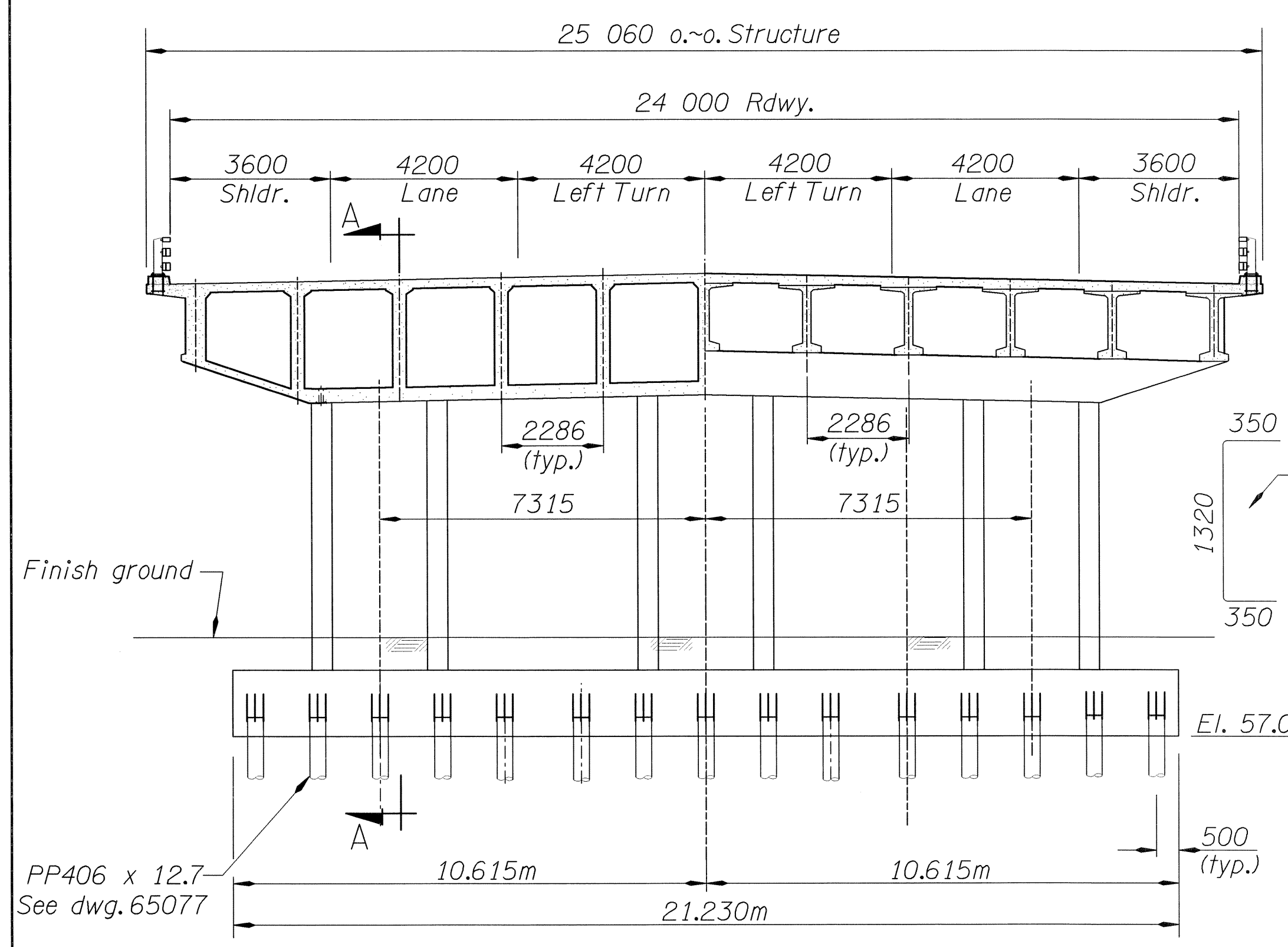
PLAN BENT-2
Scale 1:100



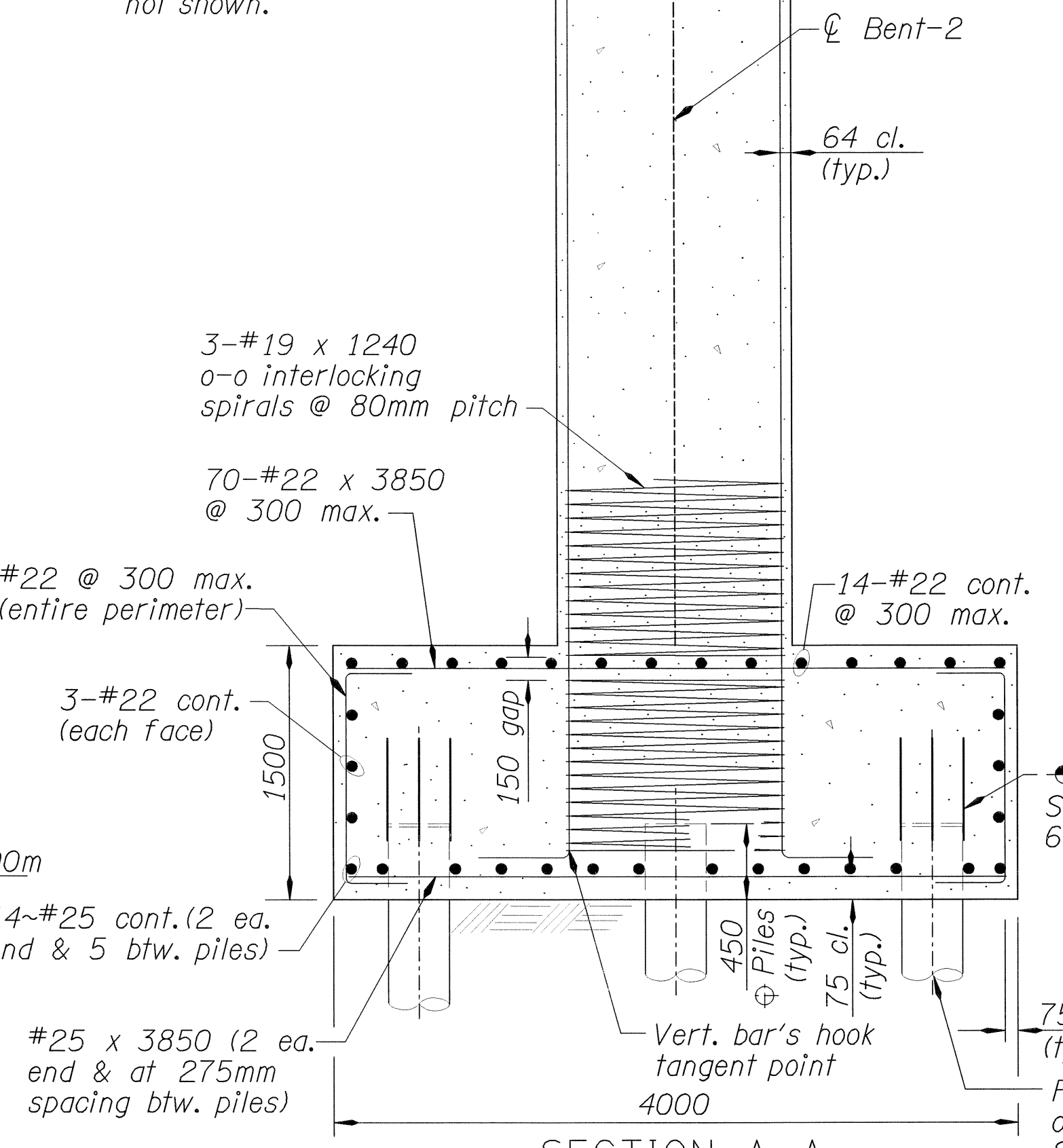
Note:
Protective fencing
not shown.



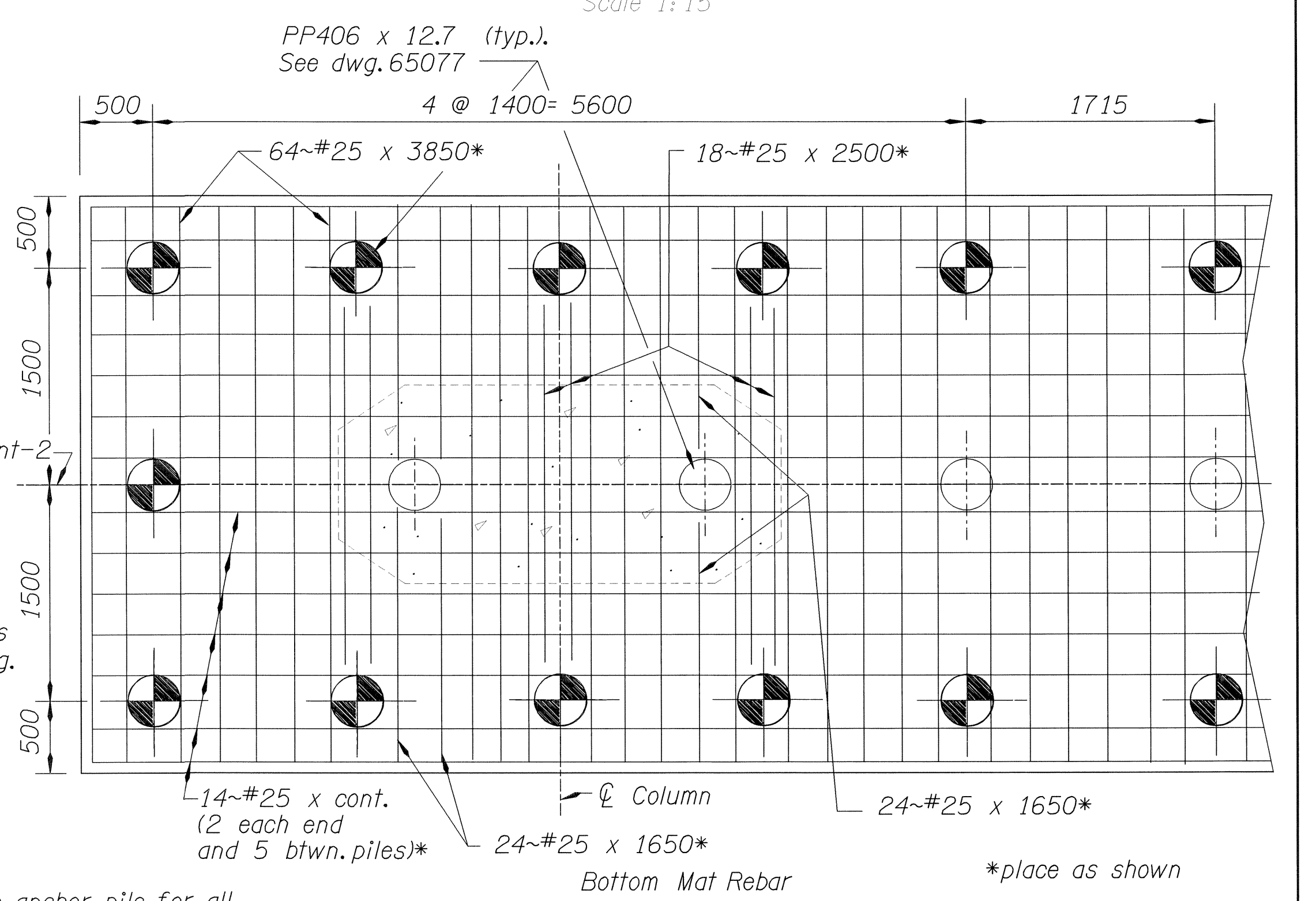
TYPICAL COLUMN SECTION
Scale 1:15



ELEVATION BENT-2
Scale 1:100



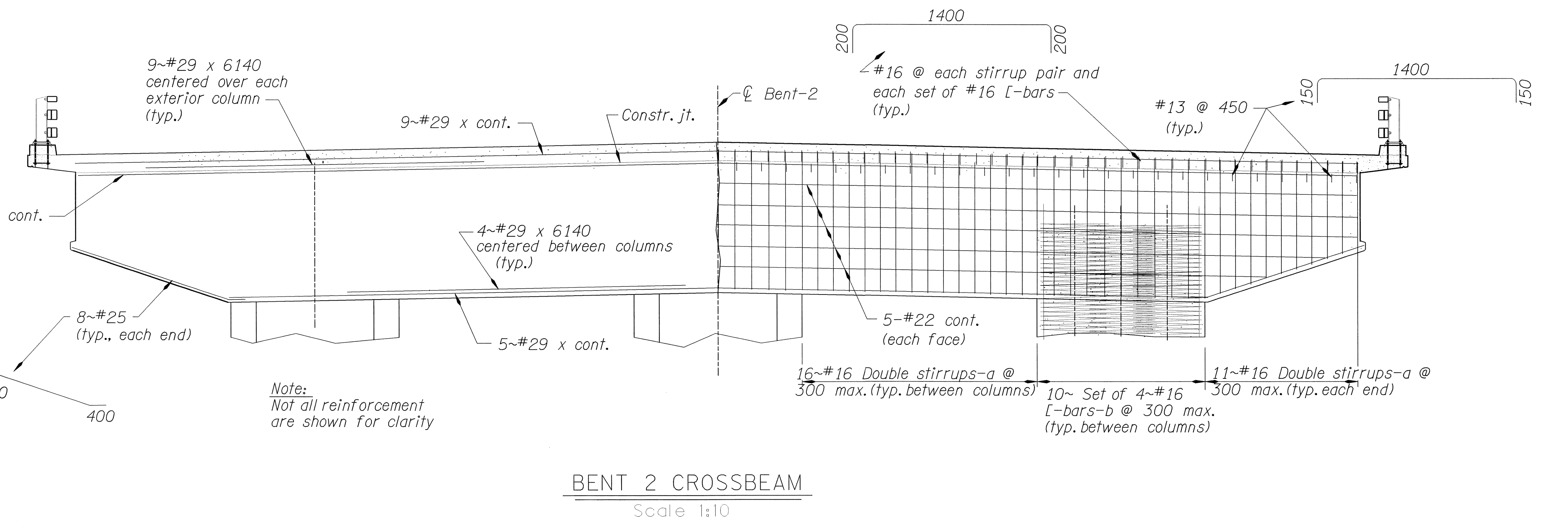
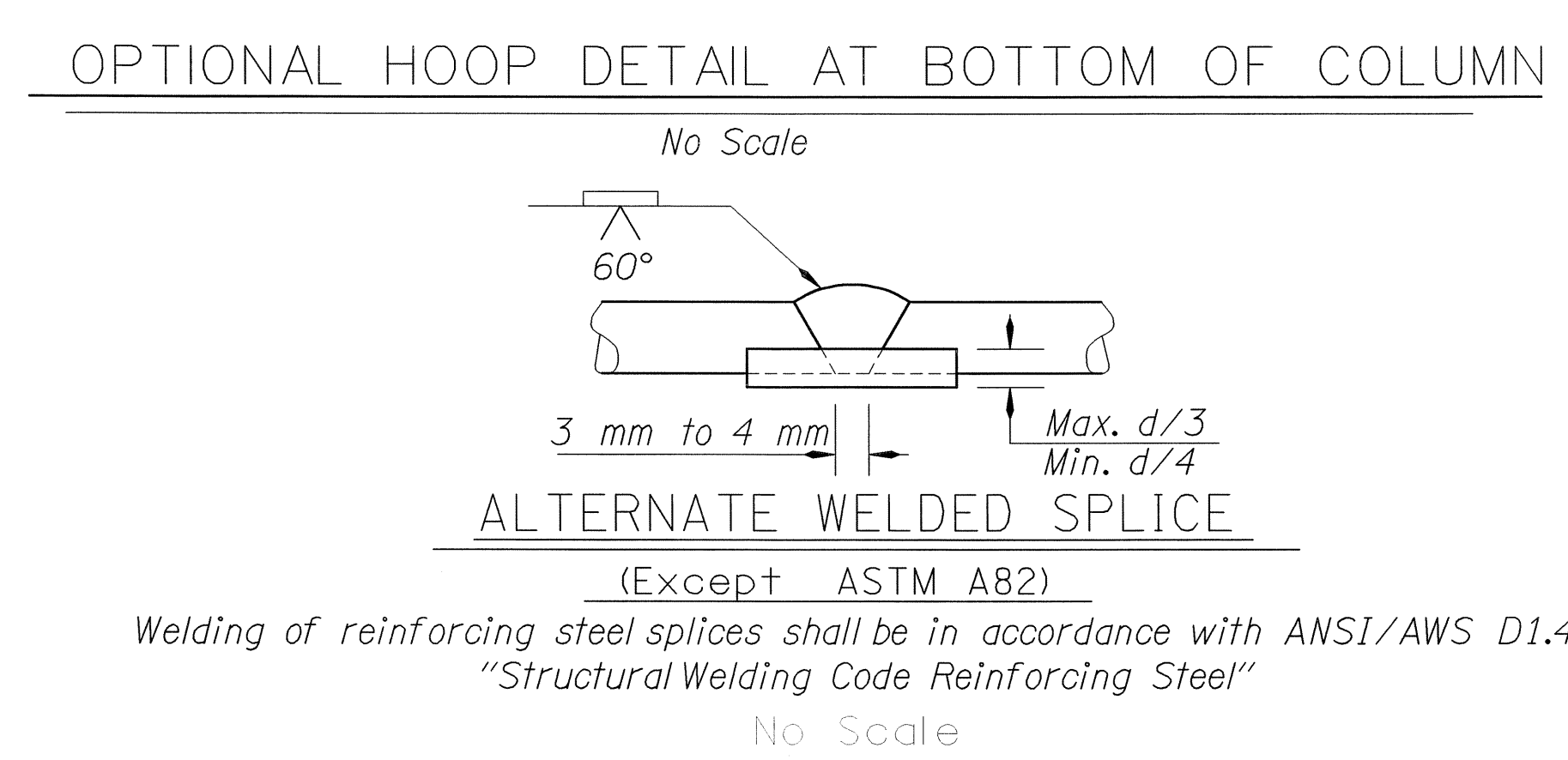
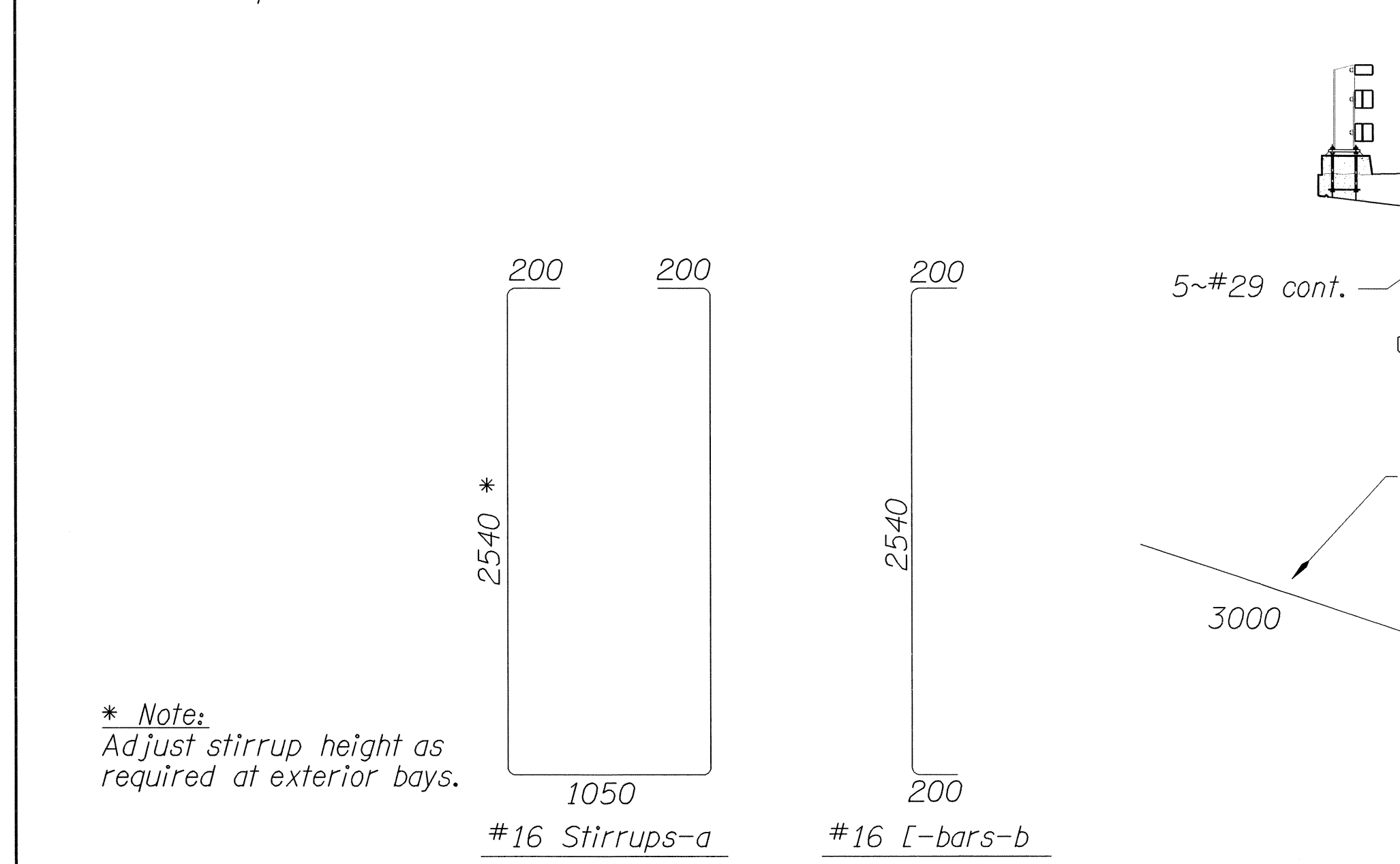
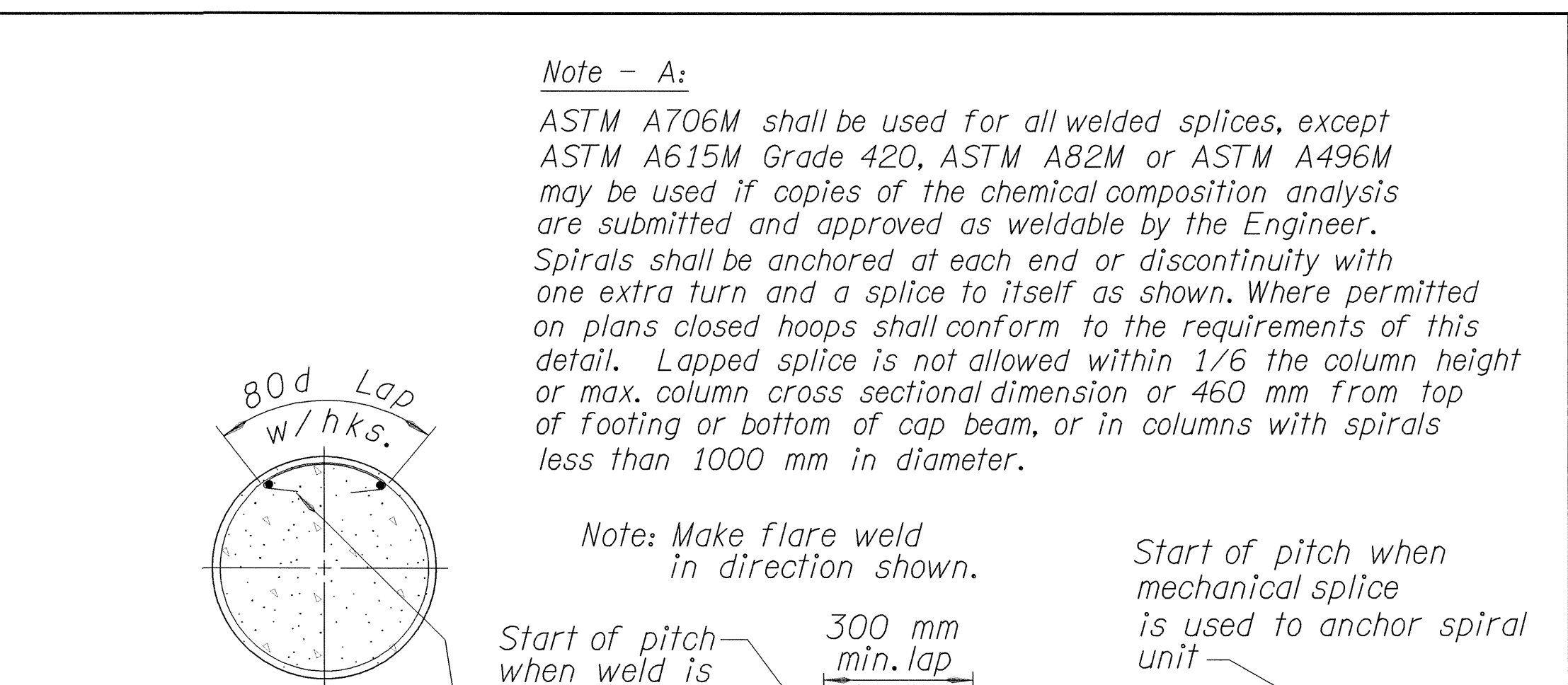
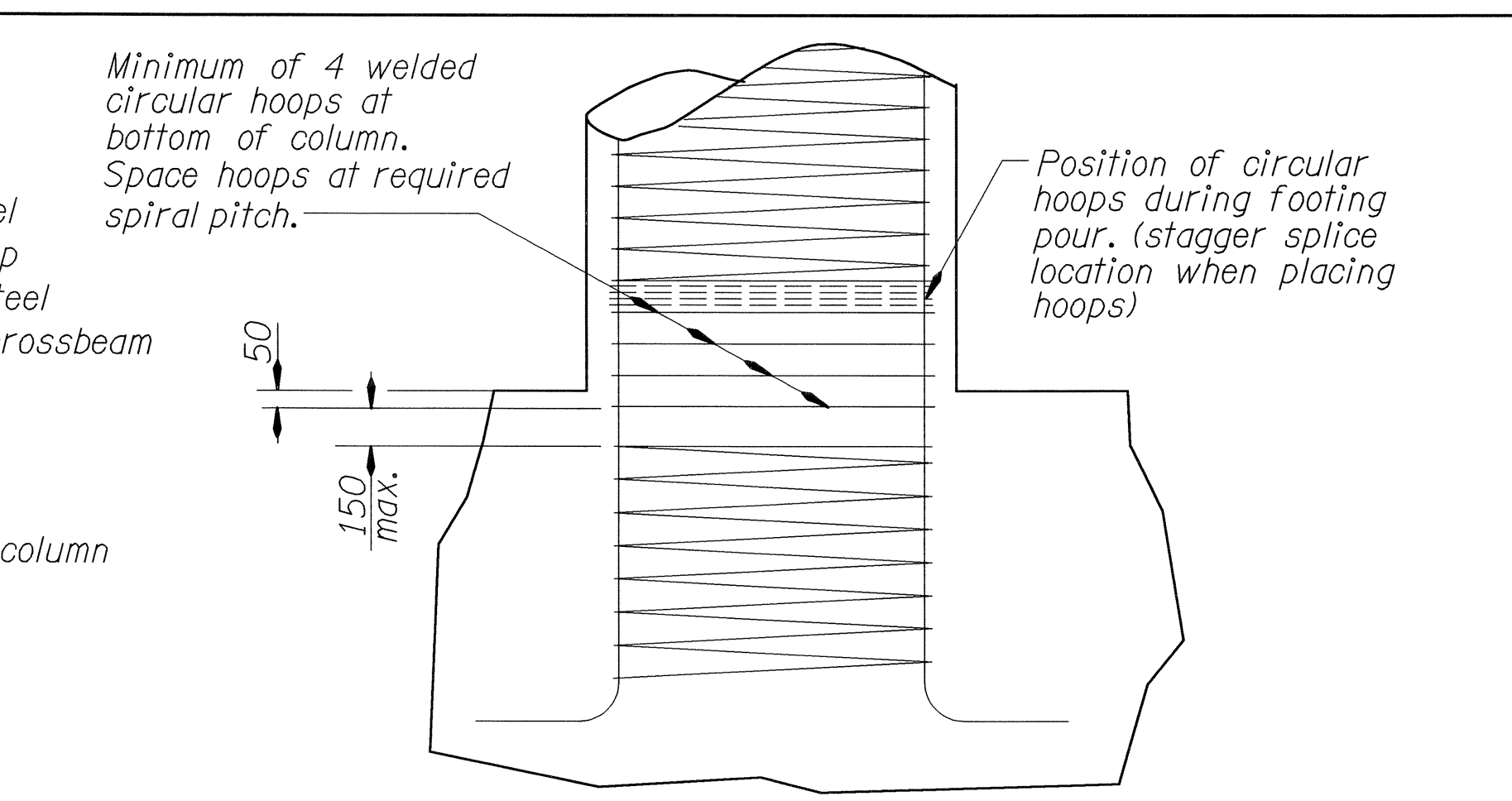
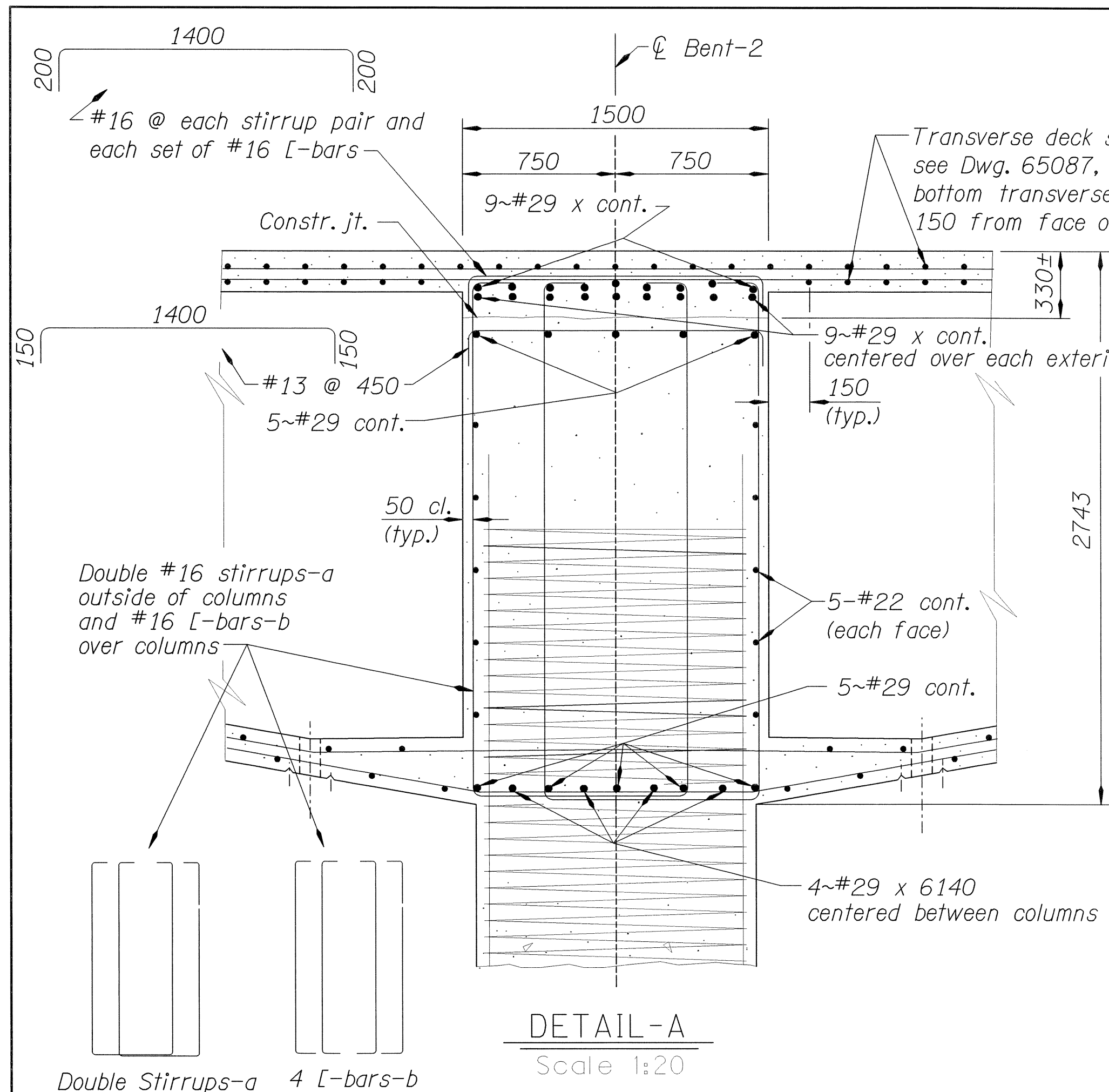
SECTION A~A
Scale 1:30



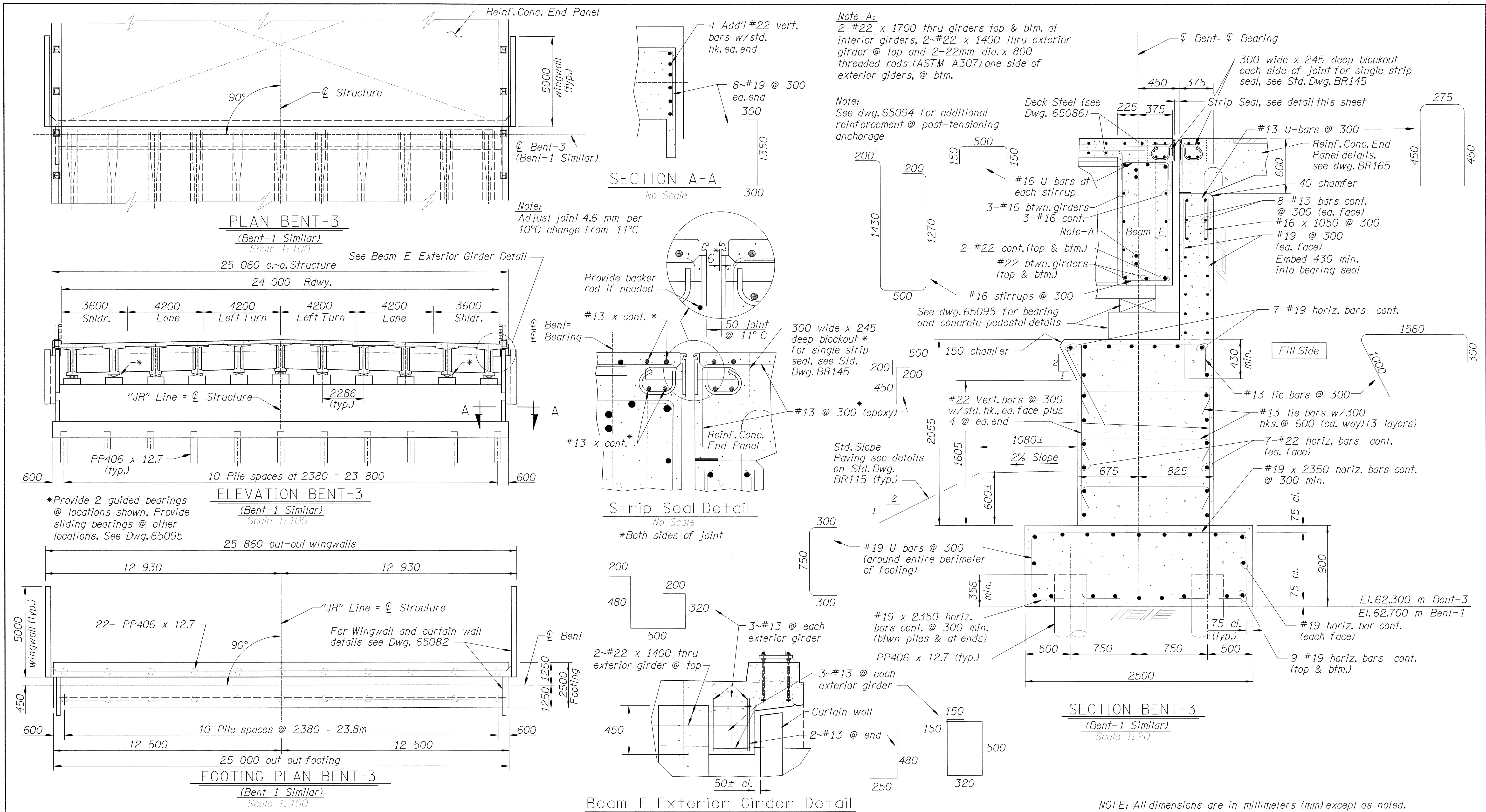
TYPICAL PILE LAYOUT
Scale 1:30

NOTE: All dimensions are in millimeters (mm) except as noted.

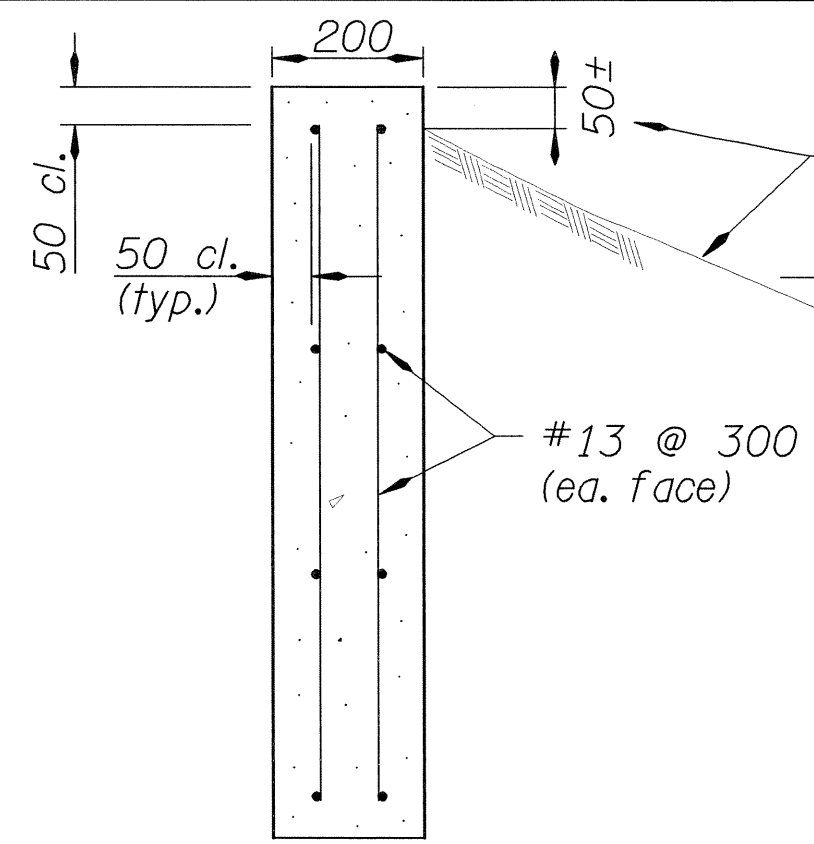
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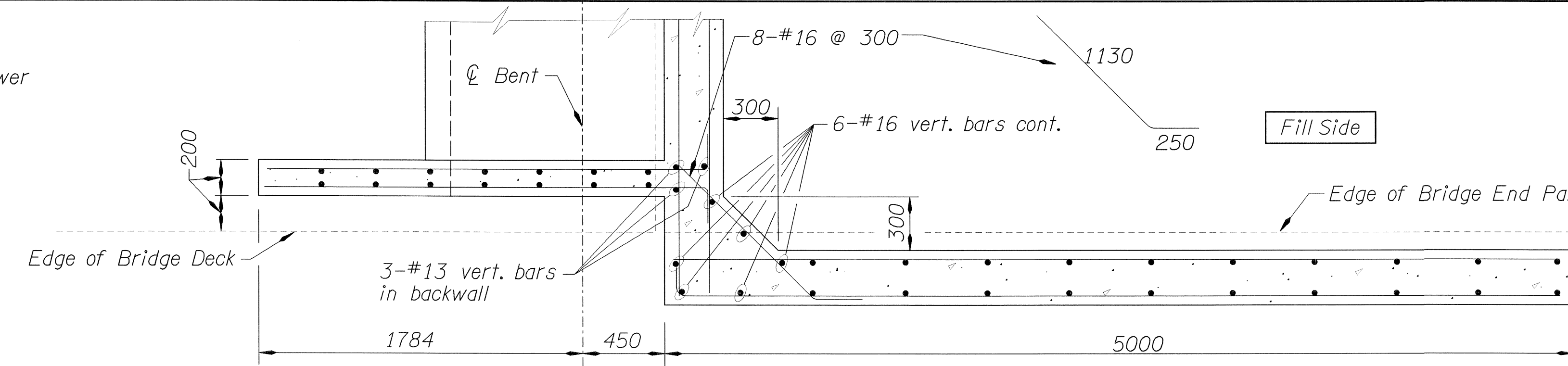
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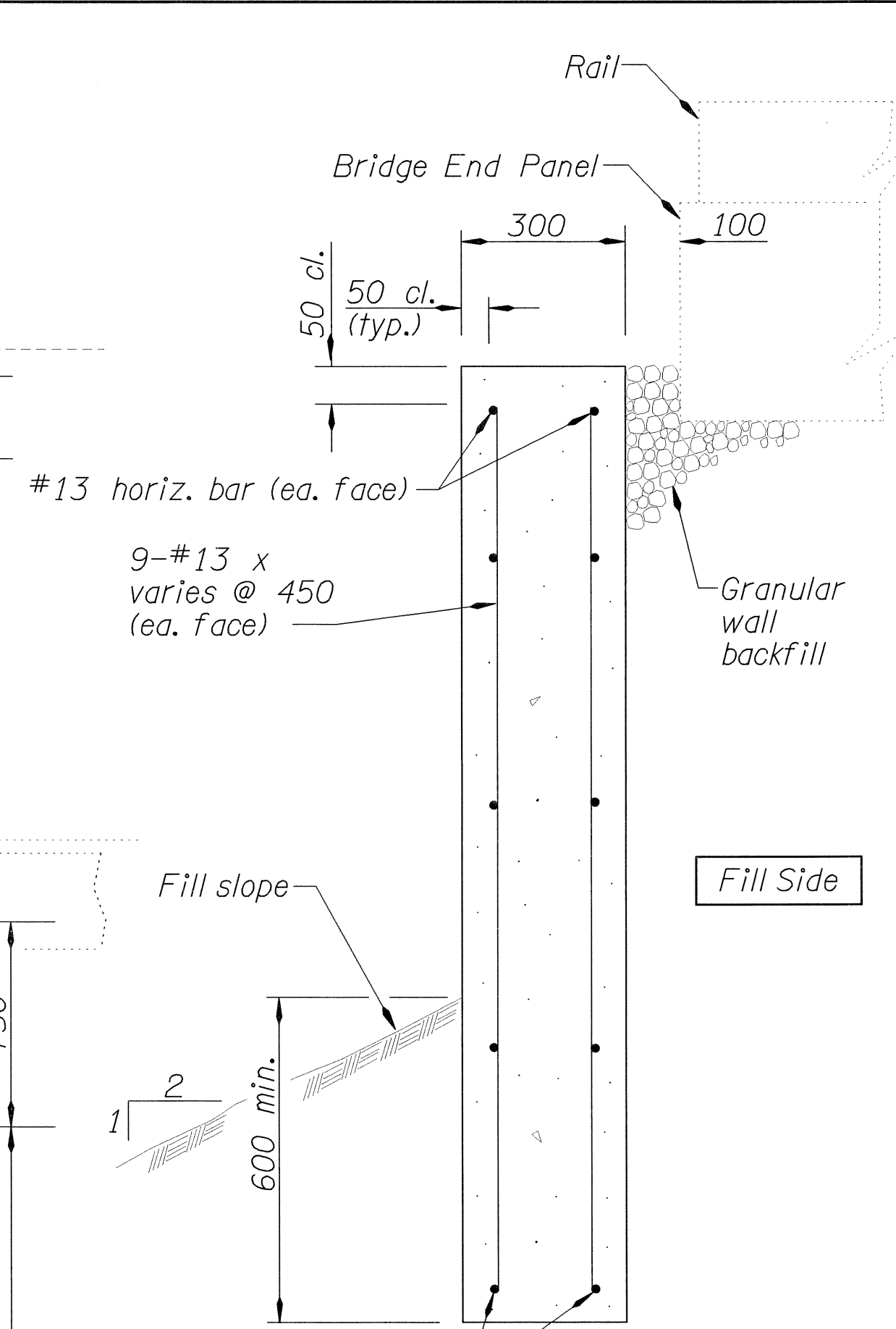
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				Phillip G. Amaya DRAFTED: Thiet Nguyen CHECKED: Hormoz Geradj REVIEWED: EXPIRES: 6-30-04	19592 DATE 19-Mar-2004 CALC. BOOK 5210-5211		6 OF 20
							DRAWING NO.
							65081



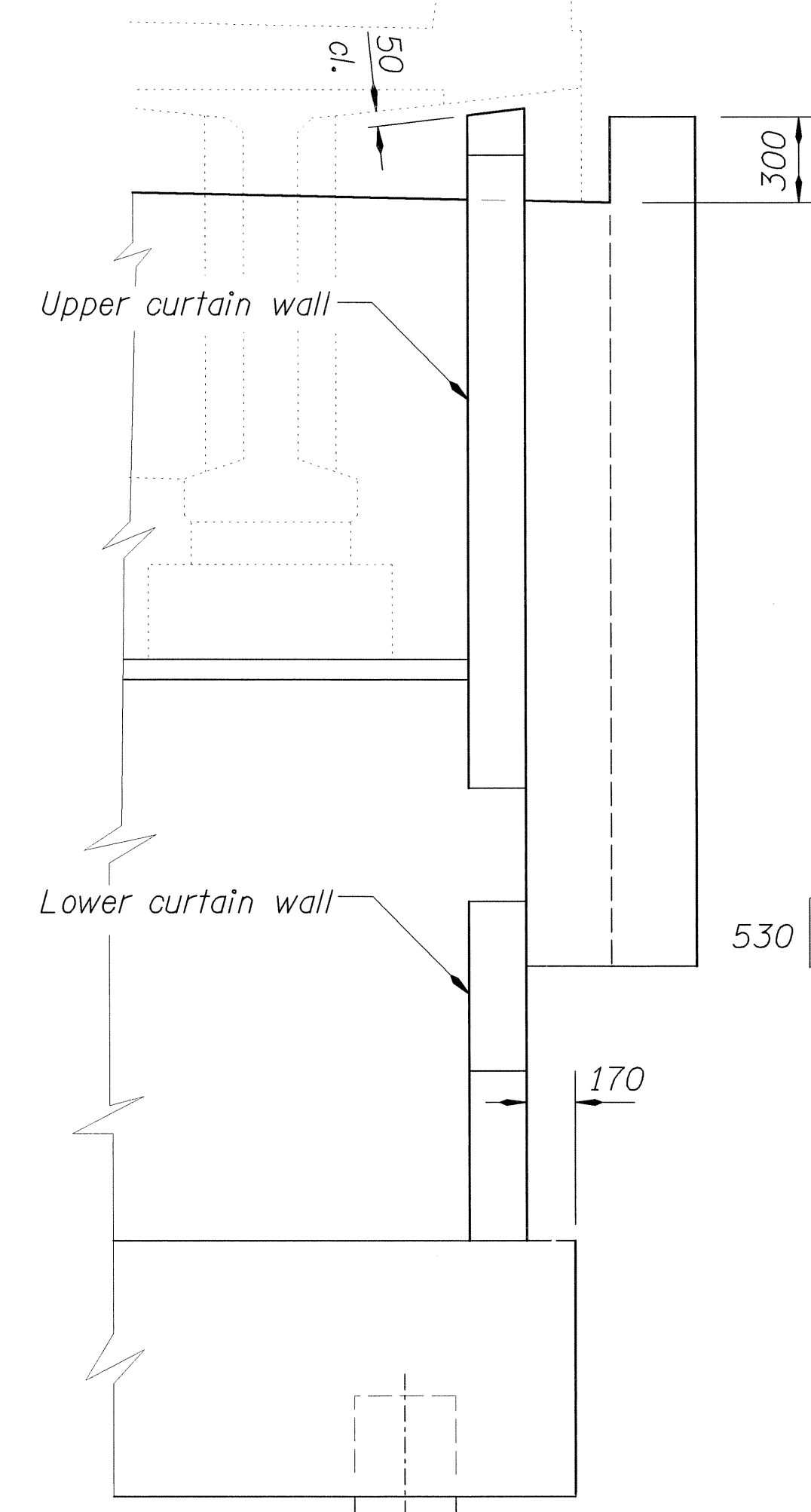
TYPICAL SECTION
Curtain Walls
Scale 1:10



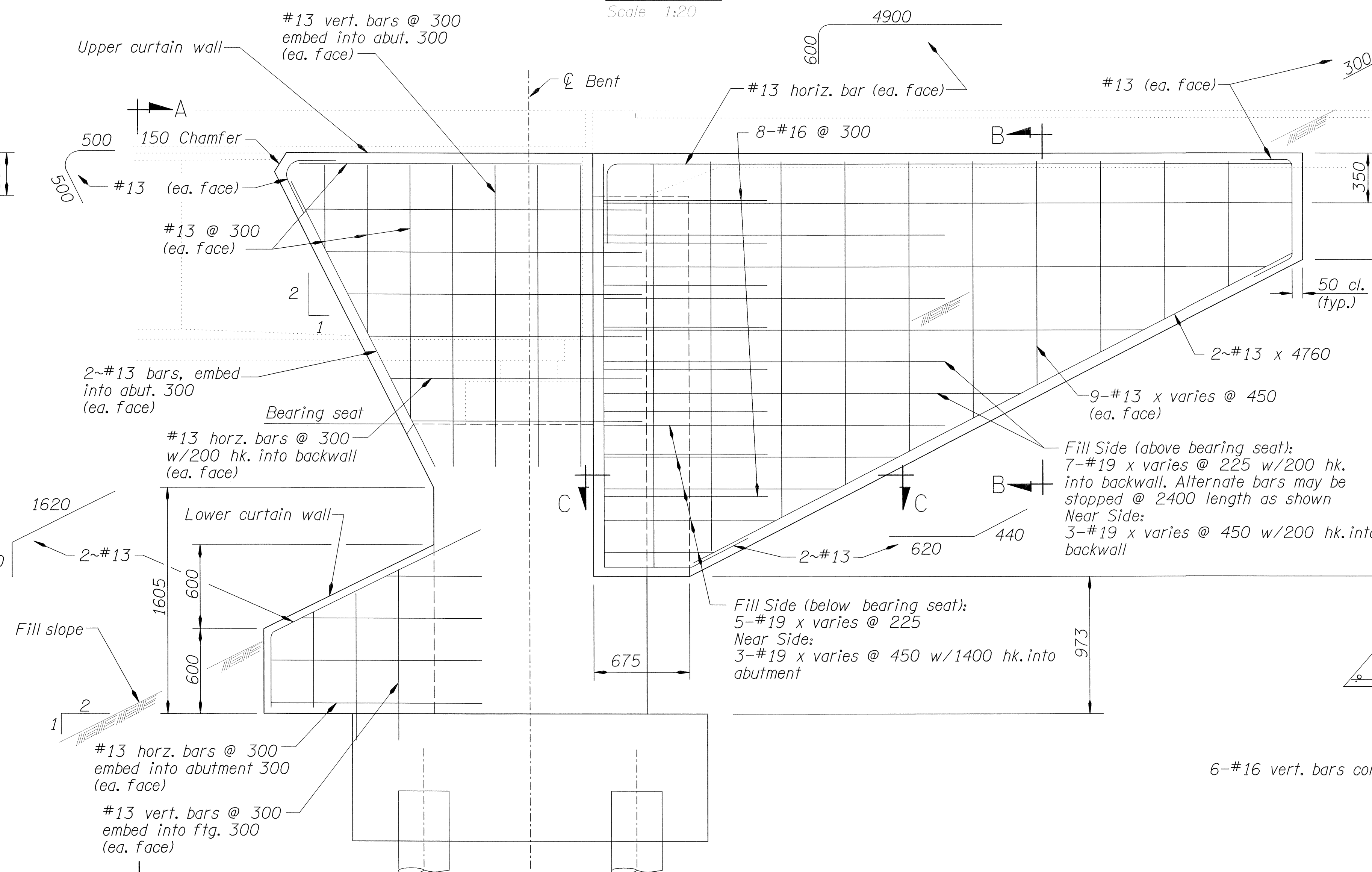
PLAN
Scale 1:20



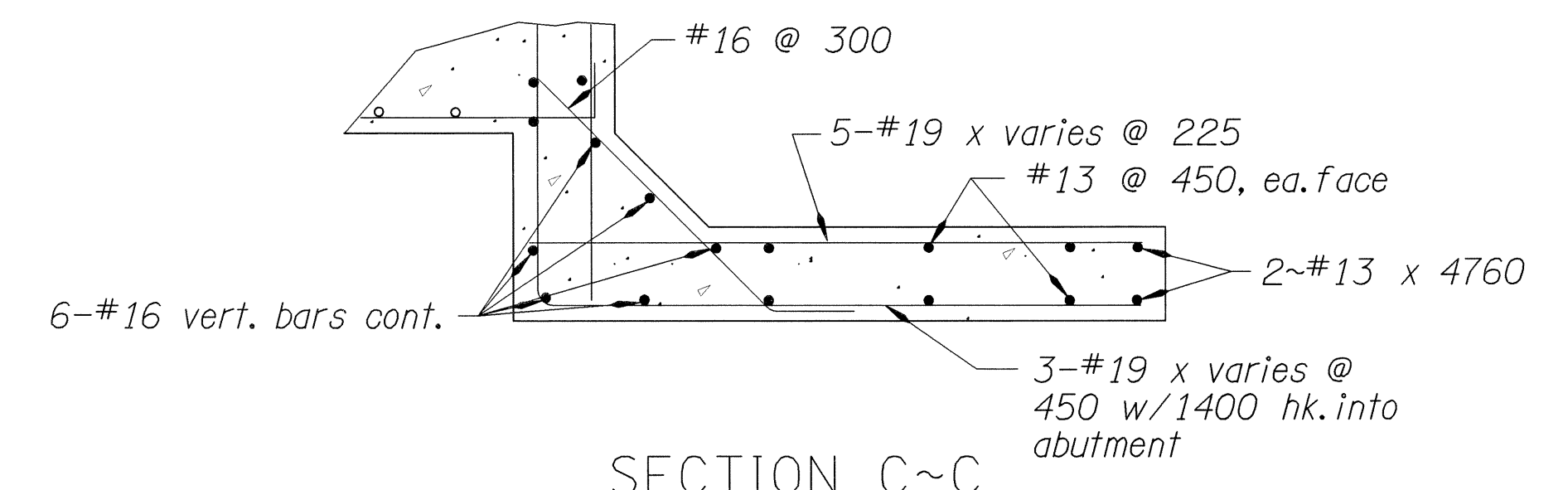
SECTION B~B
Scale 1:10



VIEW A~A
Scale 1:20



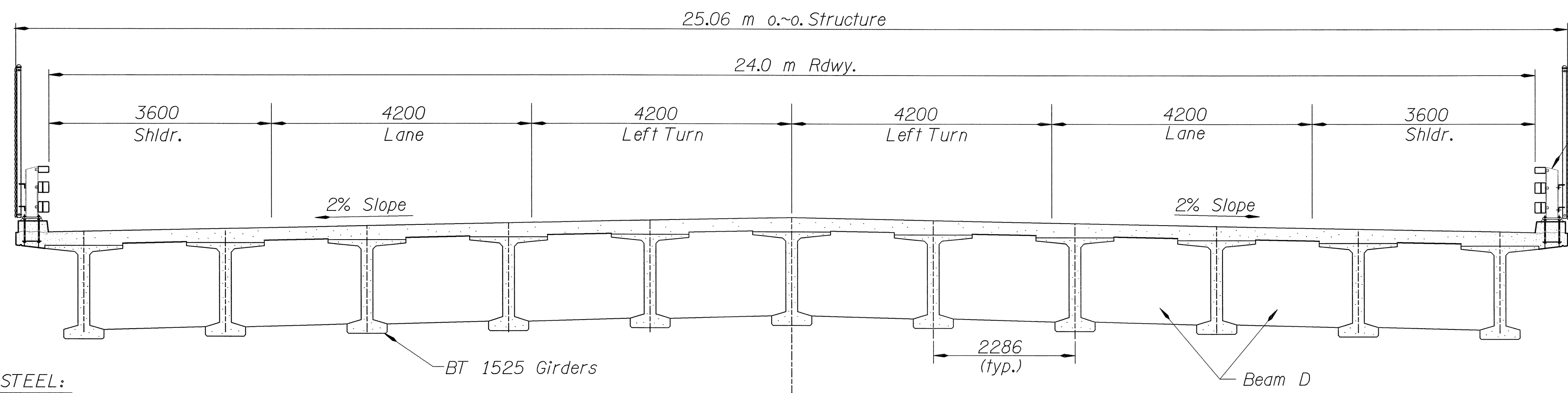
ELEVATION
Scale 1:20



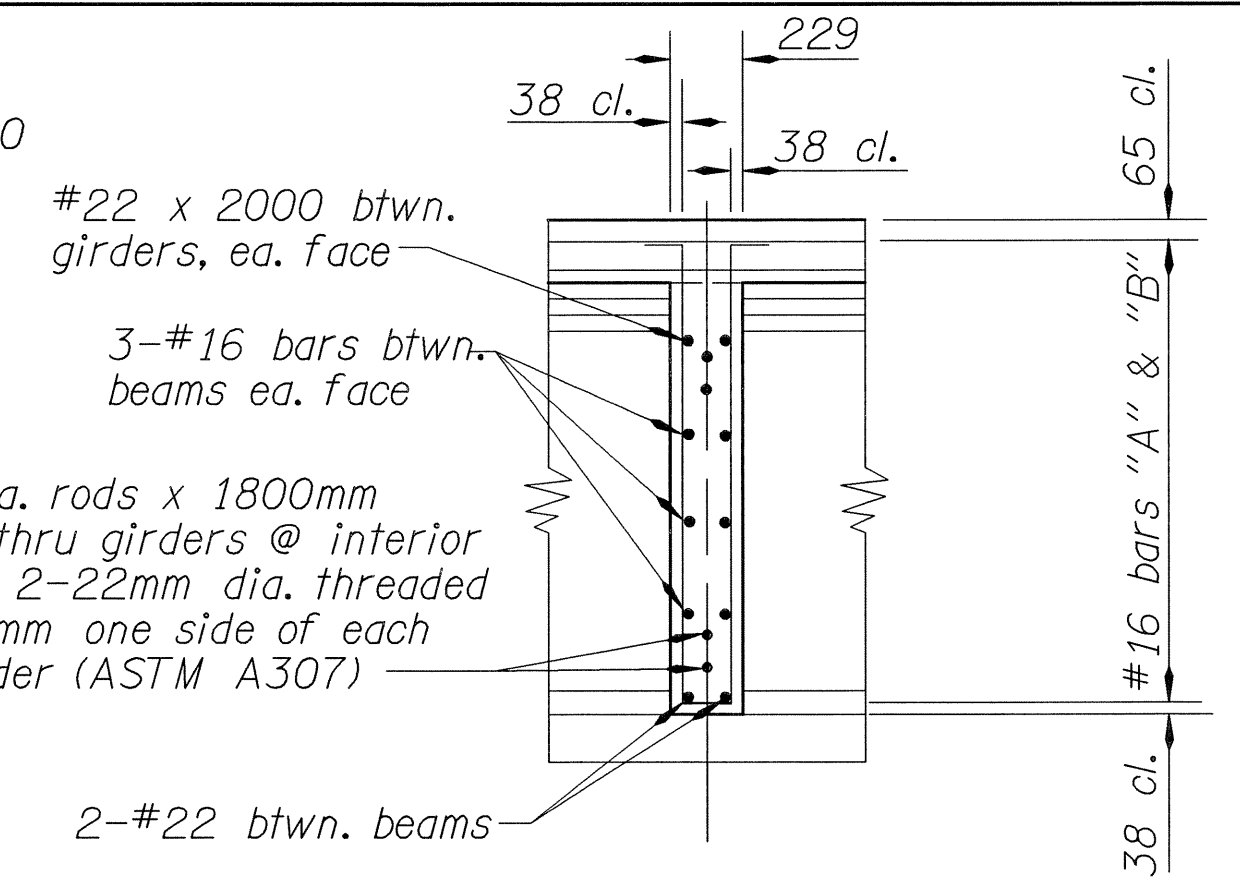
SECTION C~C
Scale 1:20

NOTE: All dimensions are in millimeters (mm) except as noted.

	DATE REVISION BY	DESIGNED: Philip G. Amaya CHECKED: Thiet Nguyen REVIEWED: Hormoz Seradj	DESIGNER EXPIRES: 6-30-04		BRIDGE NO. 19592 DATE 19-Mar-2004 CALC. BOOK 5210-5211	JACKSON SCHOOL ROAD OVER HWY 47 WINGWALL AND CURTAIN WALL DETAILS	SHEET 7 OF 20 DRAWING NO. 65082
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Type "D" Protective fencing each side. See Std. Dwg. BR240
 3-Tube Curb Mounted Rail, each side. See Std. Dwg. BR208



2-22mm dia. rods x 1800mm top & btm. thru girders @ interior girders and 2-22mm dia. threaded rods x 800mm one side of each exterior girder (ASTM A307)

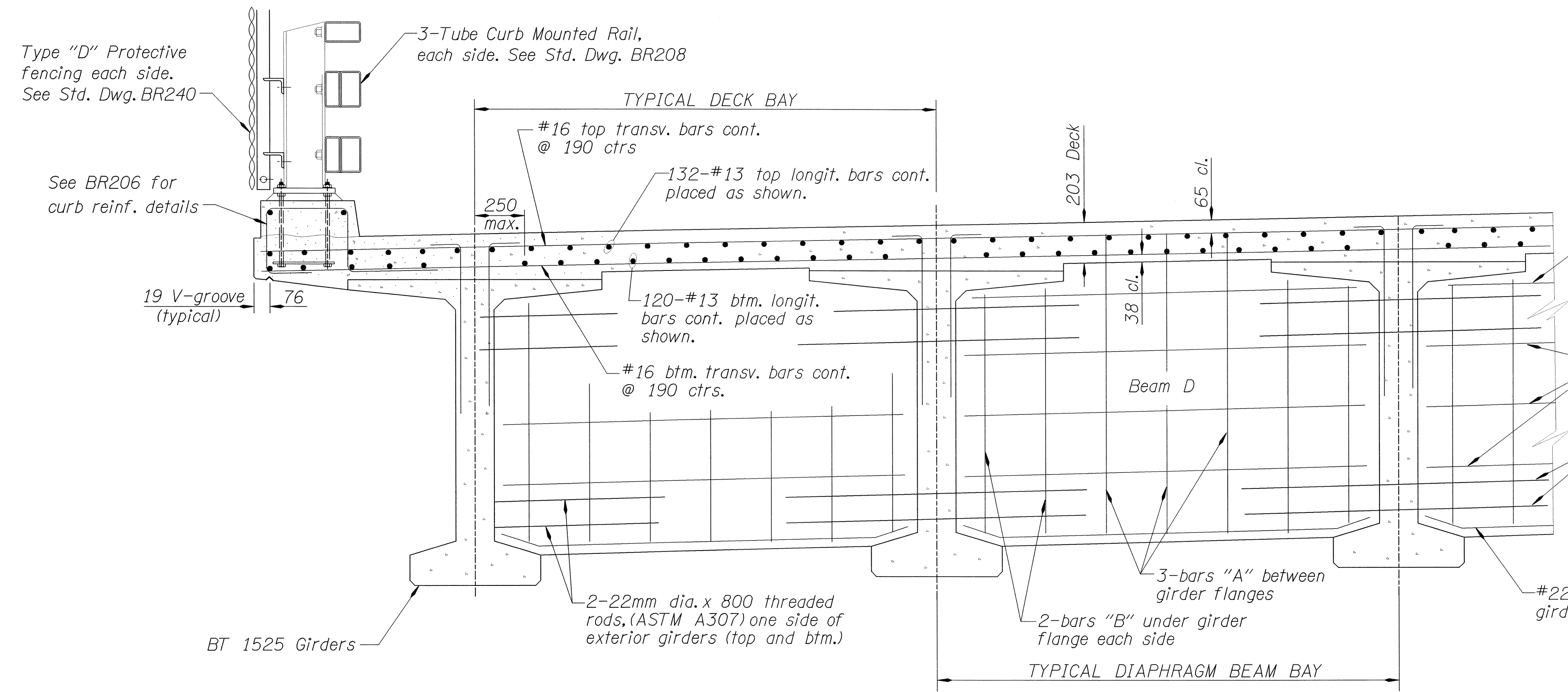
TYPICAL - BEAM "D"
 No Scale

DECK STEEL:

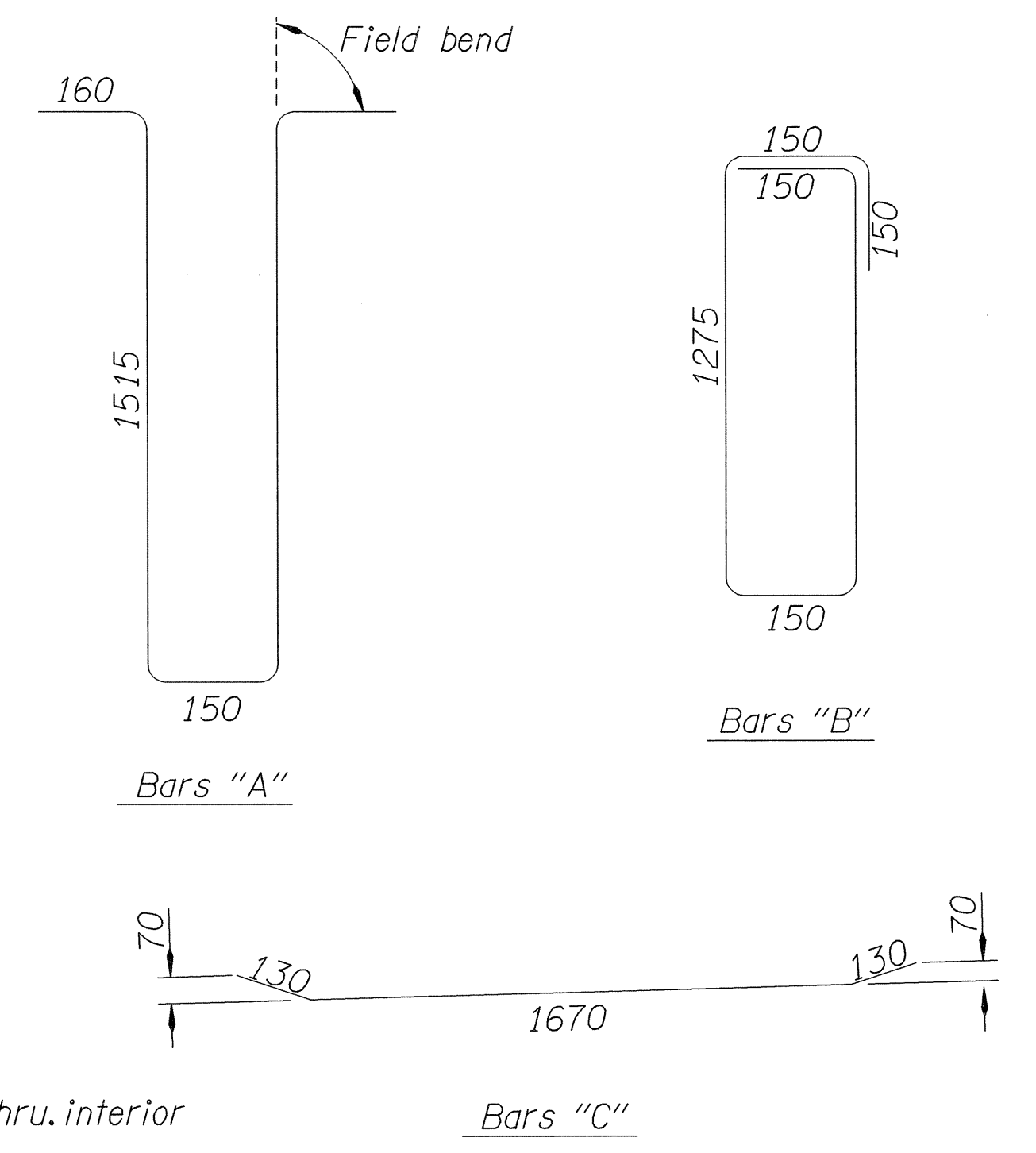
Top Mat
 132-#13 longit. bars cont. placed as shown
 #16 transverse bars @ 190 ctrs.
 Stop transverse bars 50 cl. of exp. jt. breakout

Bottom Mat
 120-#13 longit. bars cont. (11 btw. beams and 5 at overhang)
 #16 transverse bars at 190 ctrs.
 Stop transverse bars 50 clear of end beam face.

TYPICAL DECK SECTION
 AT PRECAST GIRDERS
 Scale: 1:50



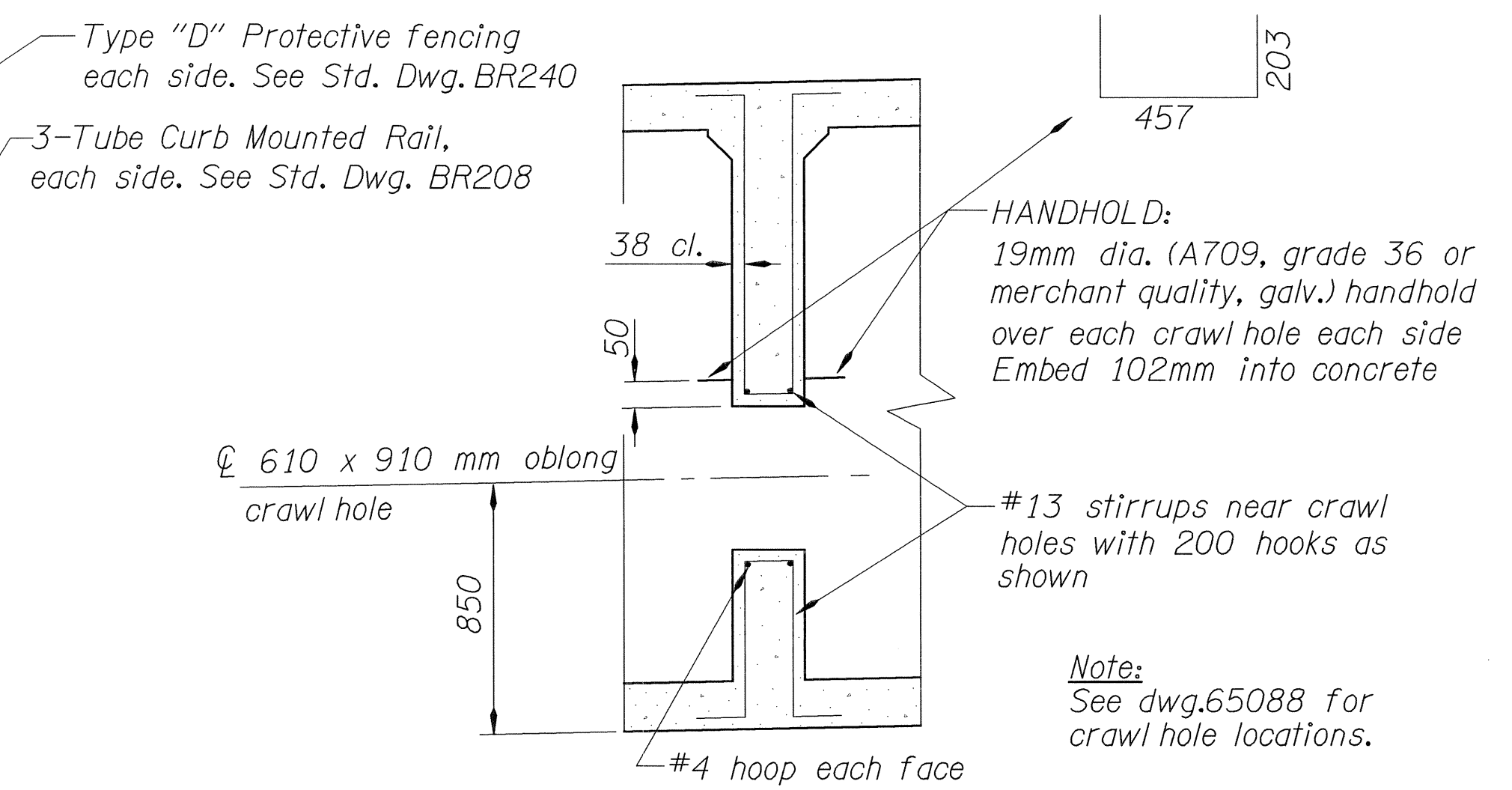
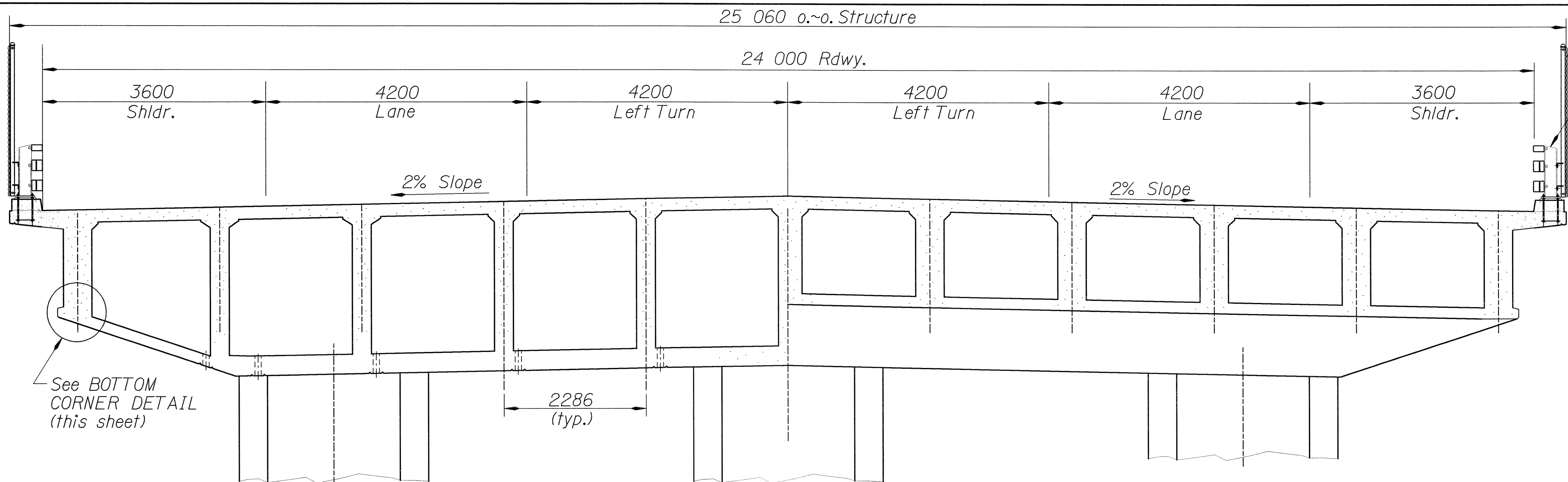
#22 x 2000 btwn. girders, ea. face
 3-#16 dia. x 2000 btwn. girders, ea. face
 2-22mm dia. rods x 1800 thru interior girders, top and btm. (typ.)



PARTIAL DECK SECTION
 Scale: 1:15

NOTE: All dimensions are in millimeters (mm) except as noted.

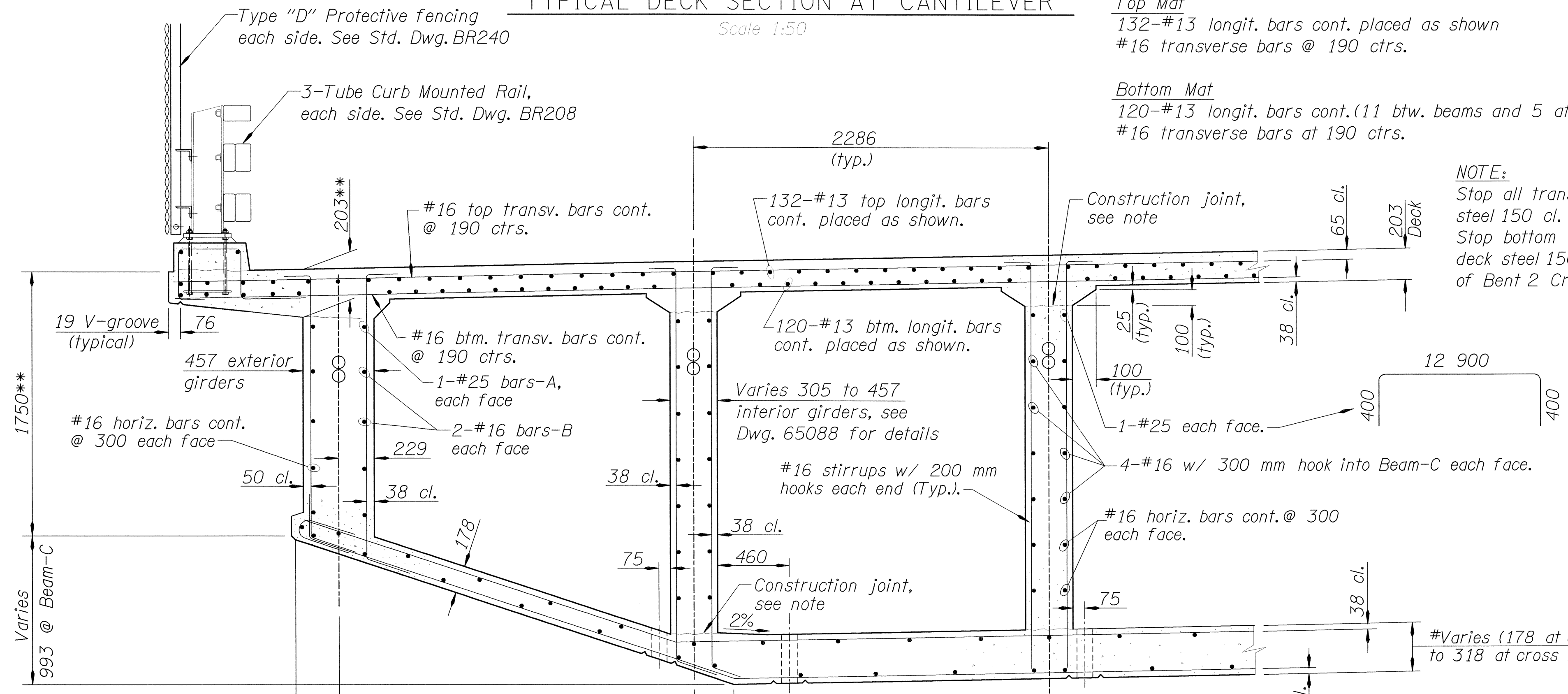
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				Philip G. Amaya	19592			
				DRAFTED: <i>Philip G. Amaya</i>	DATE			19-Mar-2004
				CHECKED: <i>Thiet Nguyen</i>	CALC. BOOK			5210-5211
			REVIEWED: <i>Hamaz Seradj</i>	DESIGNER		DECK SECTION AT PRECAST GIRDERS	DRAWING NO.	
				<i>Craig Shike</i>			65086	
				REGISTERED PROFESSIONAL ENGINEER 14056 OREGON JULY 26, 1998 CRAIG SHIKE				
				EXPIRES: 6-30-04				



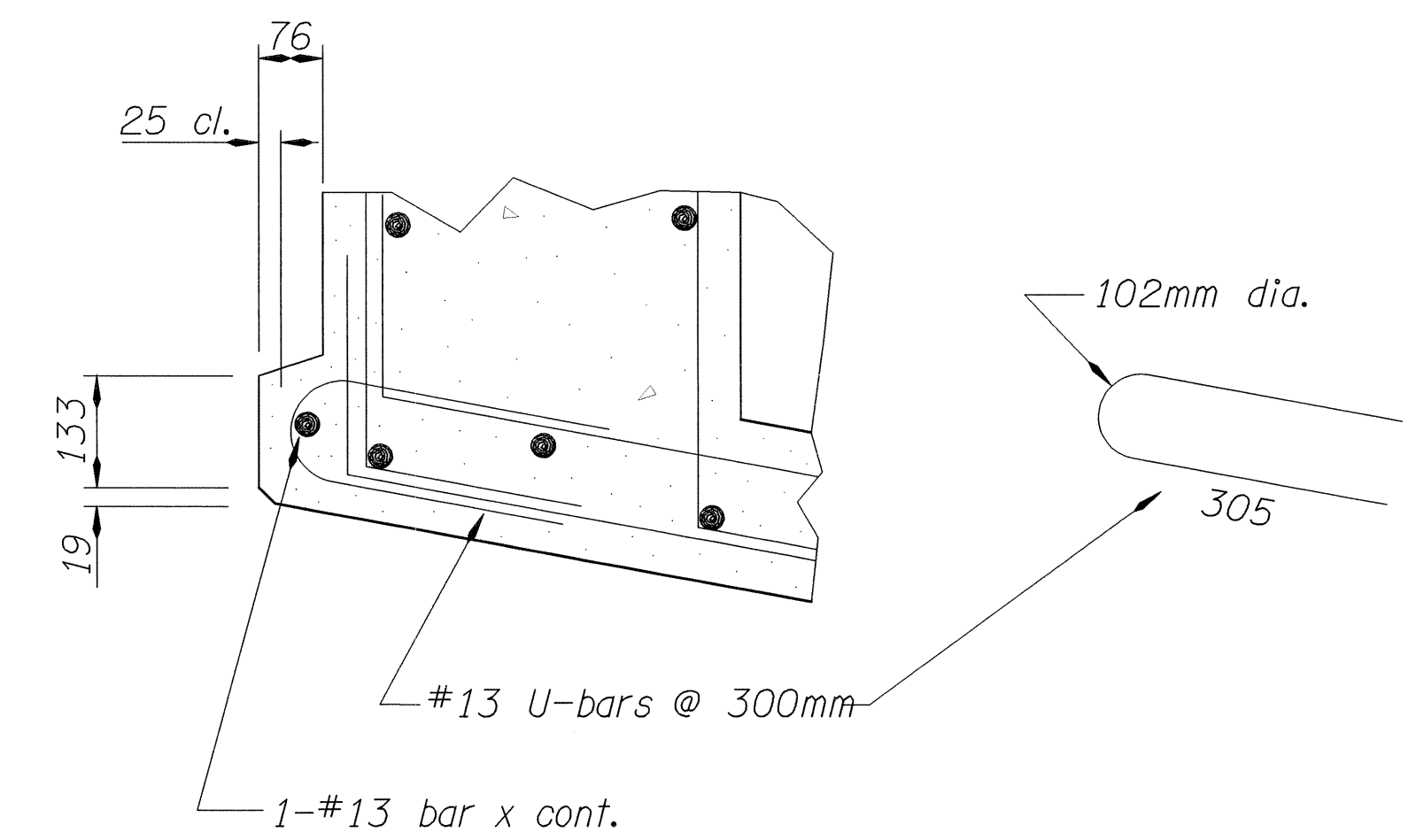
TYPICAL GIRDER AT CRAWL HOLE
No Scale

TYPICAL DECK SECTION AT CANTILEVER

DECK STEEL :
Top Mat
 132-#13 longit. bars cont. placed as shown
 #16 transverse bars @ 190 ctrs.
Bottom Mat
 120-#13 longit. bars cont. (11 btw. beams and 5 at overhang)
 #16 transverse bars at 190 ctrs.



NOTE:
 Stop all transverse deck steel 150 cl. of Beam C.
 Stop bottom mat transverse deck steel 150 cl. of Bent 2 Crossbeam.



BOTTOM CORNER DETAIL
No Scale

NOTE:
 Stop transverse bottom slab steel 150 cl. of Beam C and Bent 2 Crossbeam.

BTM. SLAB STEEL

Top Mat
 51- #16 longit. cont. as shown.
 #16 transverse straight bars @ 350 ctrs. w/300 hook into exterior stem.
Bottom Mat
 52- #16 longit. cont. as shown.
 #16 transverse straight bars @ 350 ctrs. w/300 hook into exterior stem.
 Alternate top and bottom layers @ 175 ctrs.

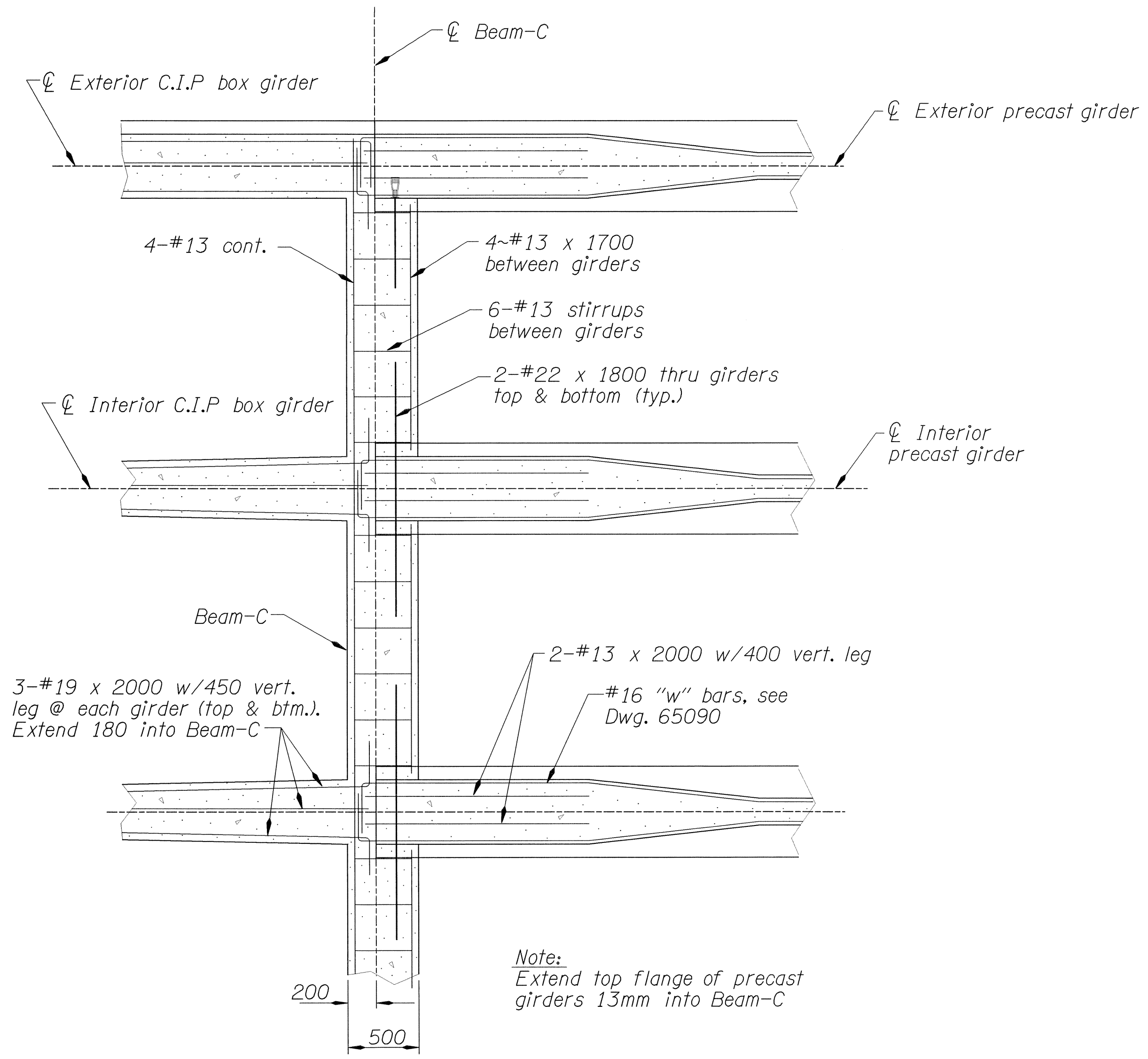
NOTE: All dimensions are in millimeters (mm) except as noted.

CONSTRUCTION JOINT NOTE:
 Provide roughened surface at construction joints. Do not trowel or smooth aggregate into space. Keep deviation from a plane surface less than 13mm

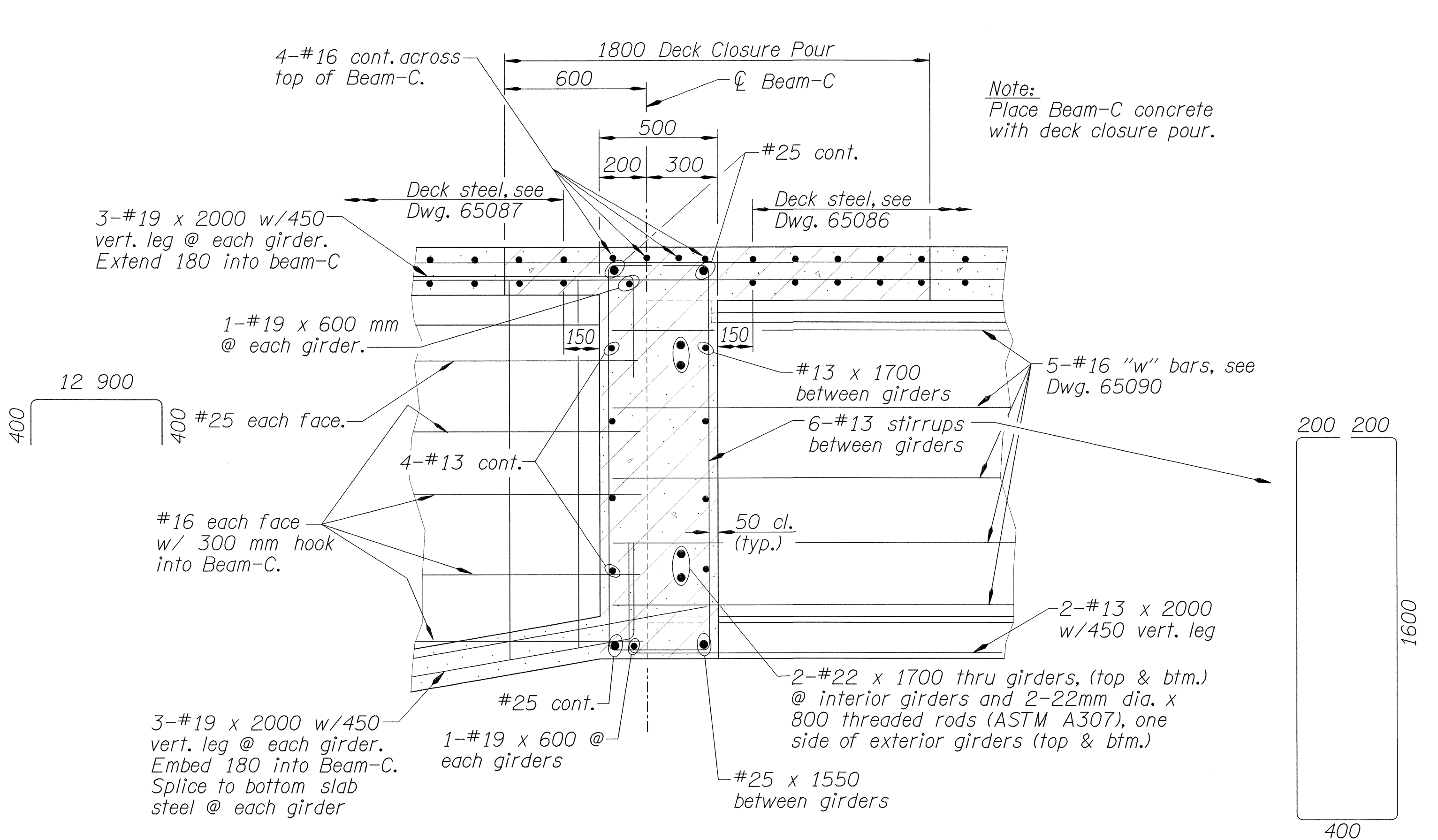
Note:
 **Adjust for actual camber in prestressed beams

PARTIAL DECK SECTION
Scale 1:20

▲	DATE	REVISION	BY	DESIGNER		BRIDGE NO.	JACKSON SCHOOL ROAD OVER HWY 47	SHEET	
				Philip G. Amaya		19592		12	
				 Thiet Nguyen Hormoz Seradj				DATE	OF
				 Hormoz Seradj EXPIRES: 6-30-04				19-Mar-2004	20
						CALC. BOOK	DECK SECTION AT CAST-IN-PLACE BOX GIRDERS	DRAWING NO.	
						5210-5211		65087	





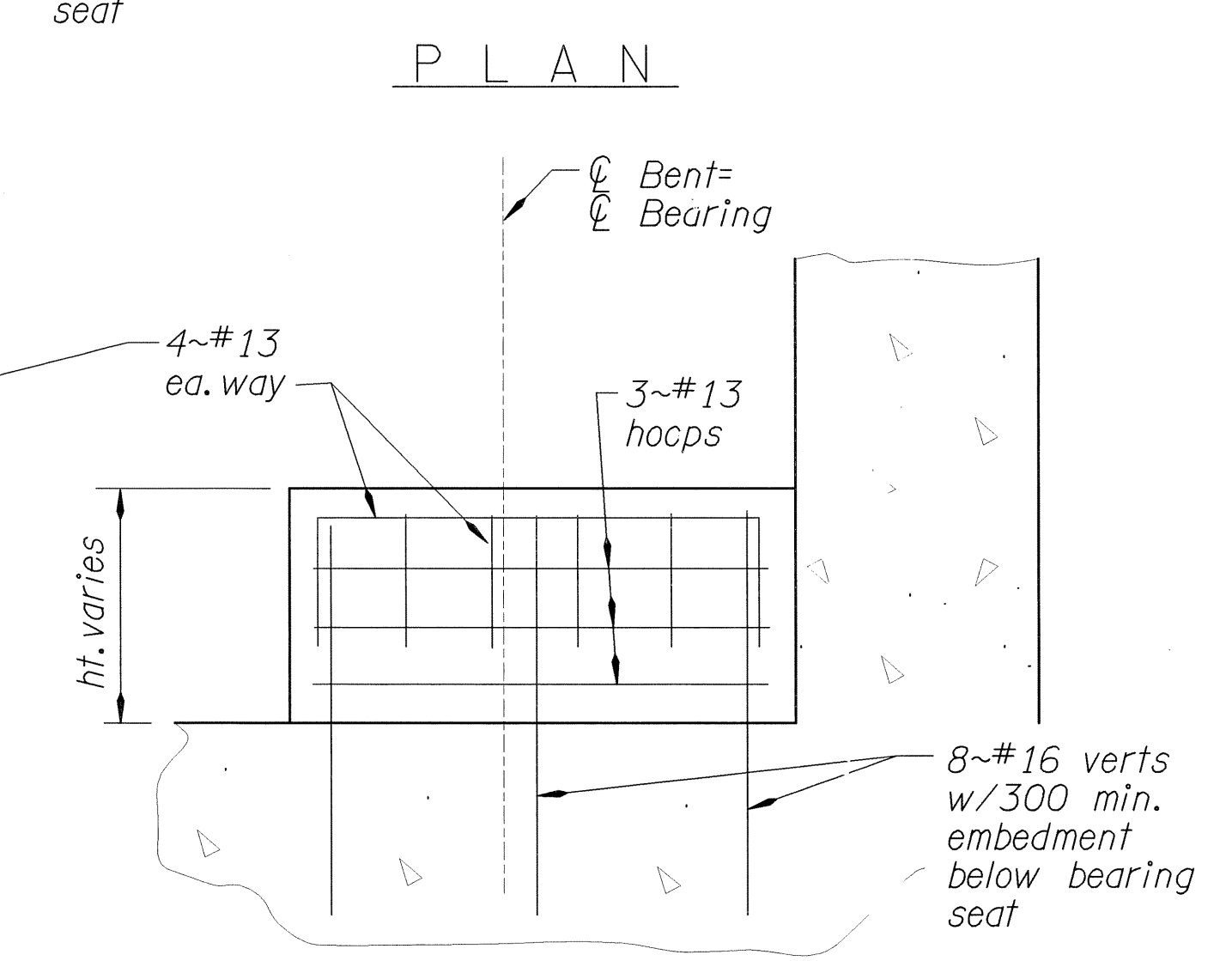
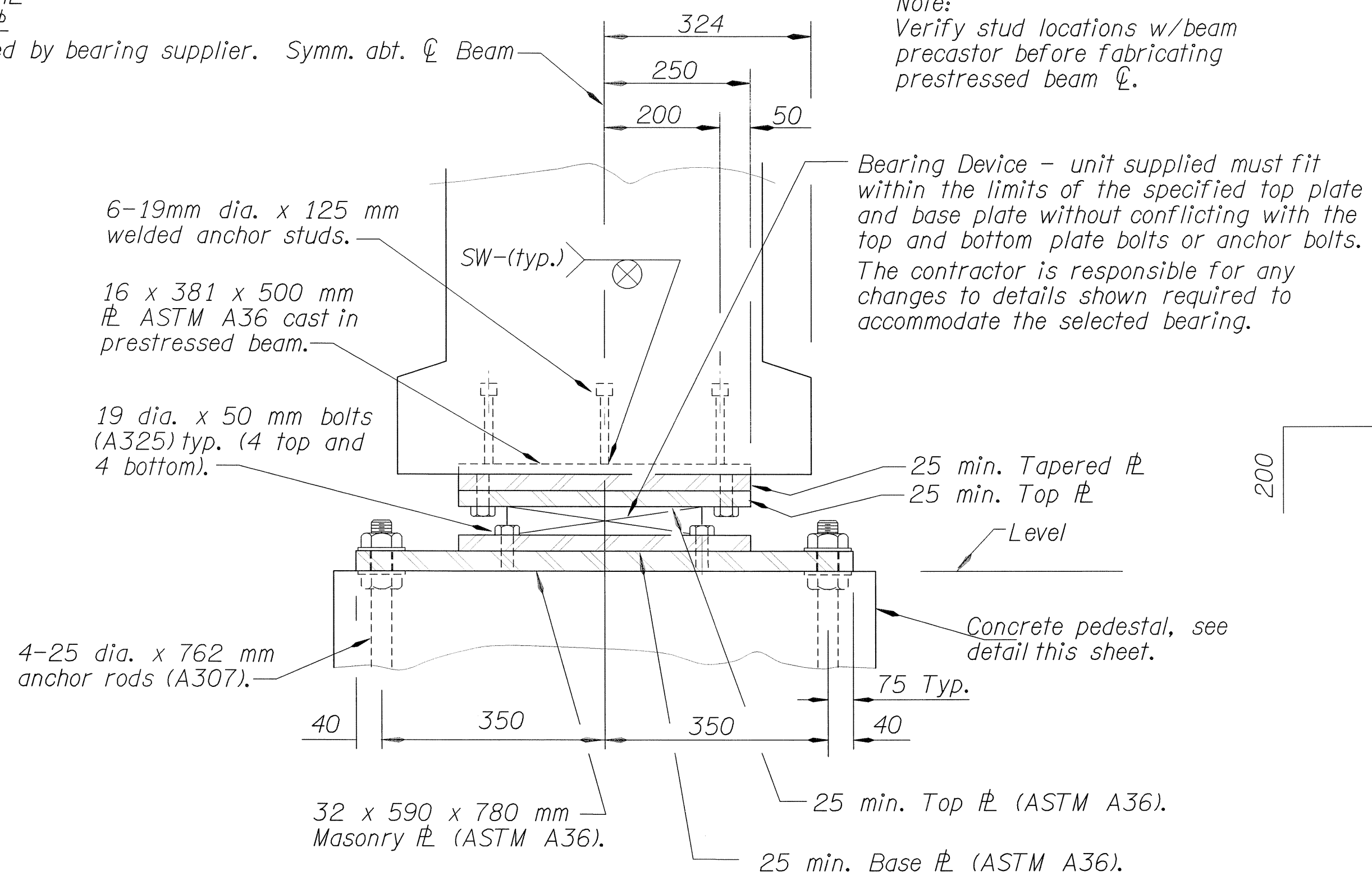
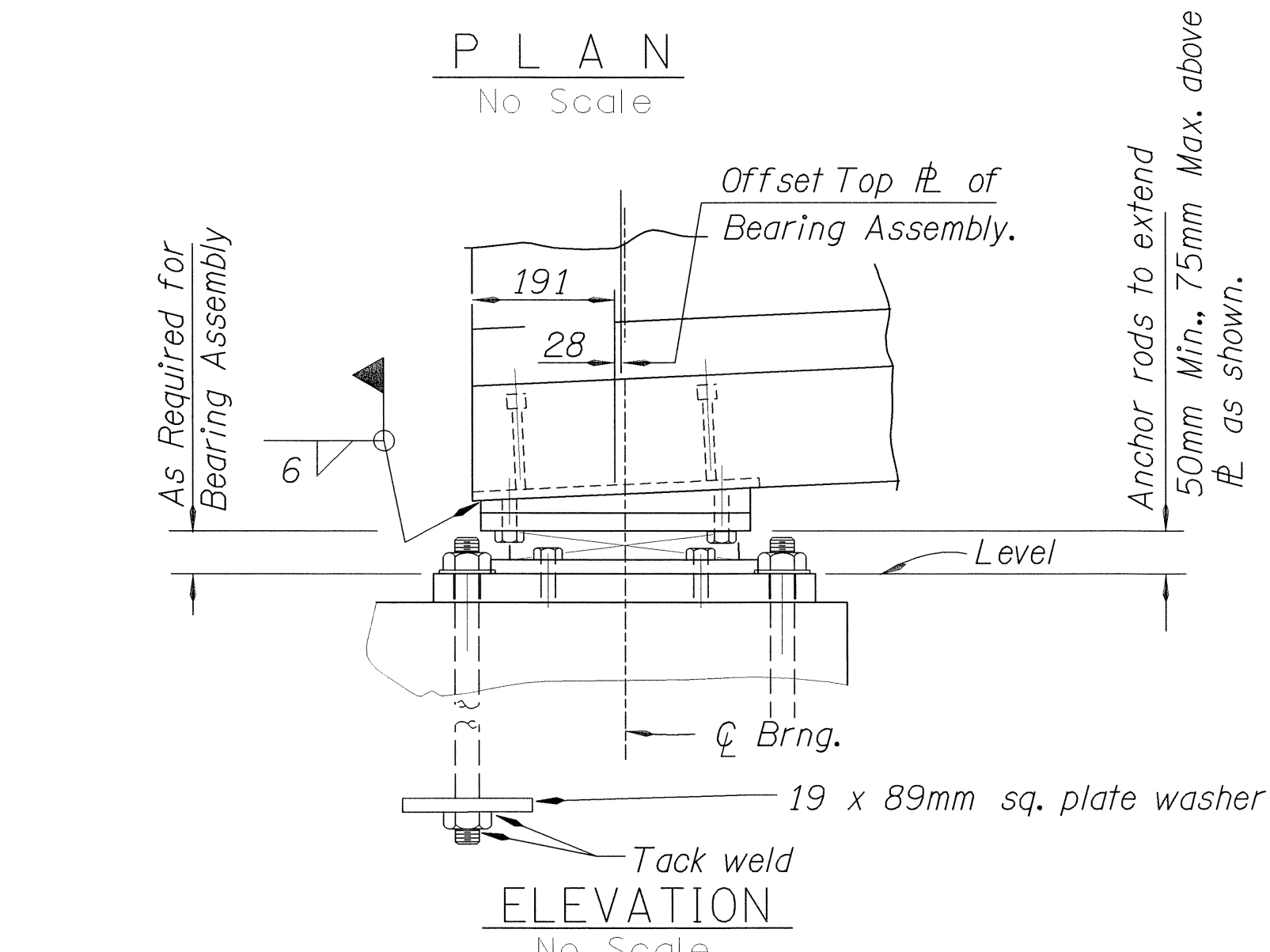
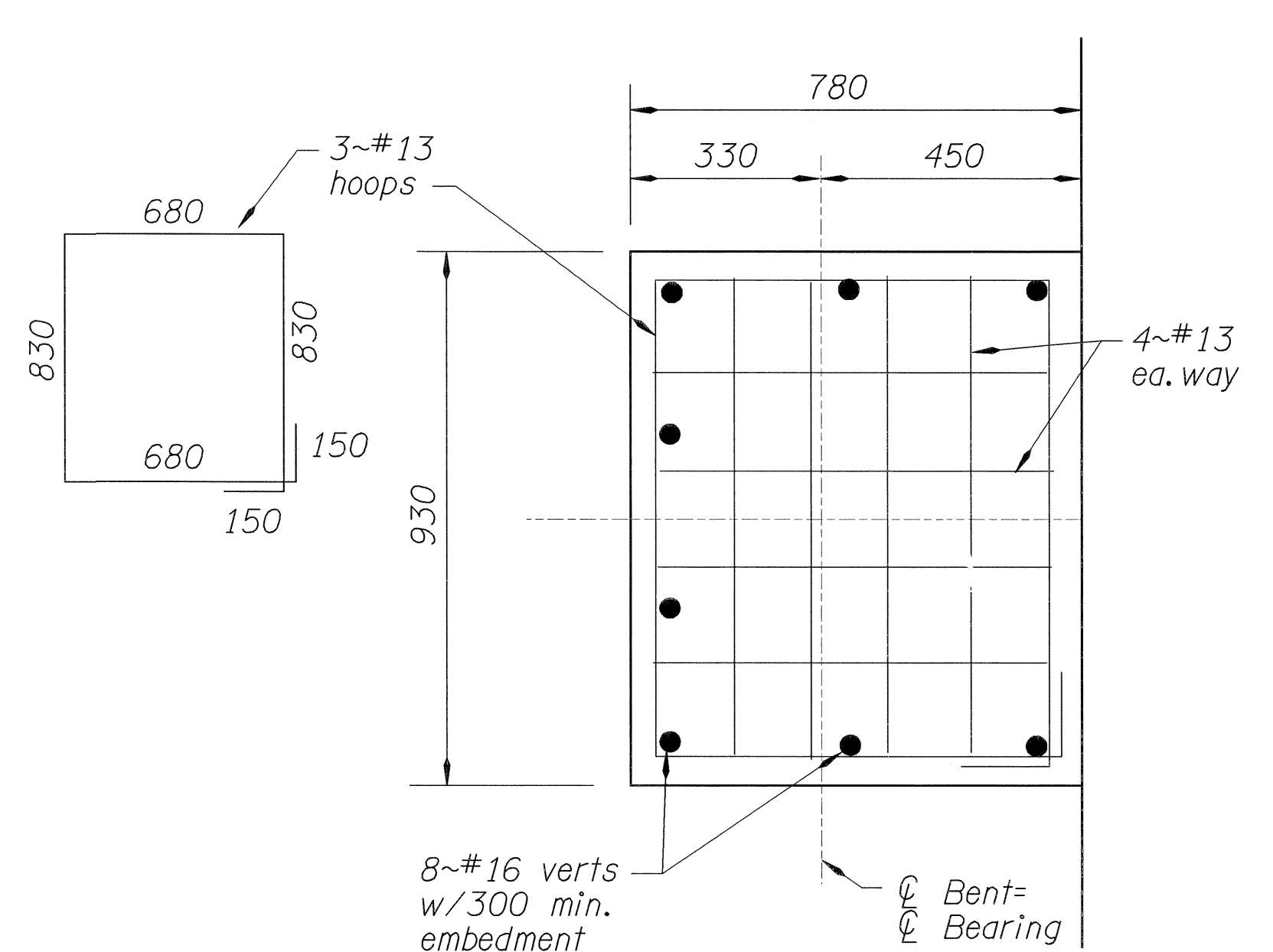
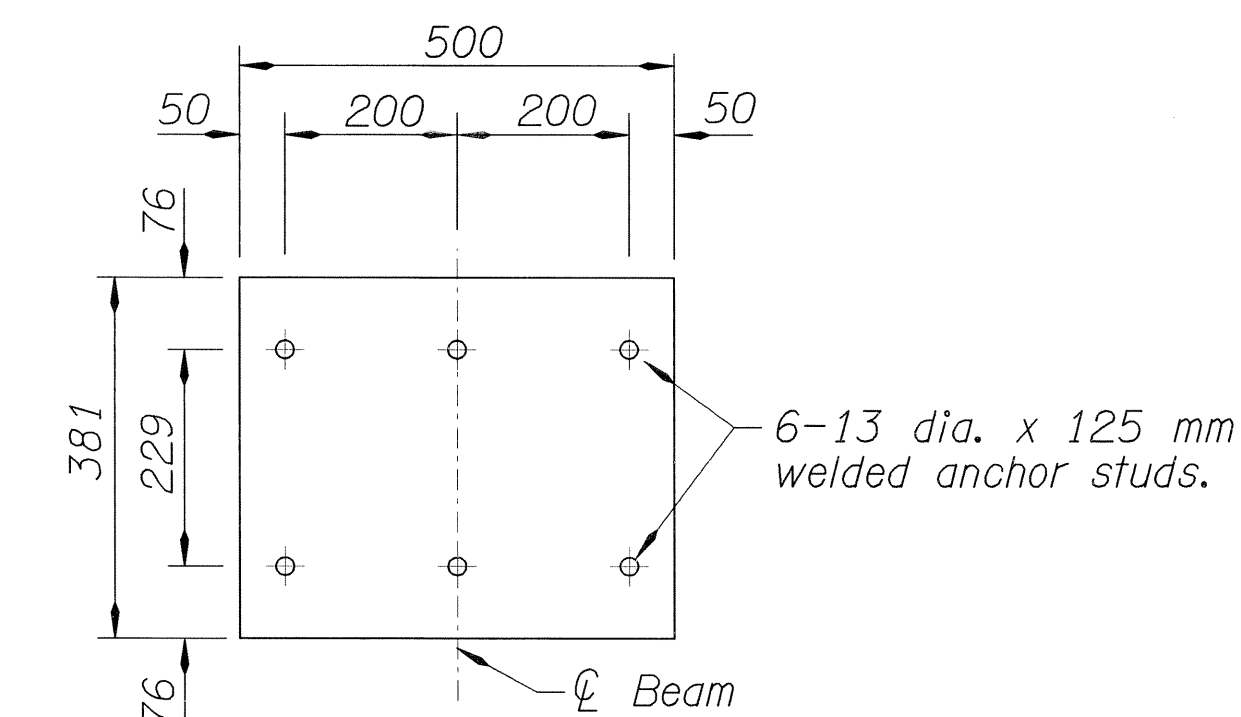
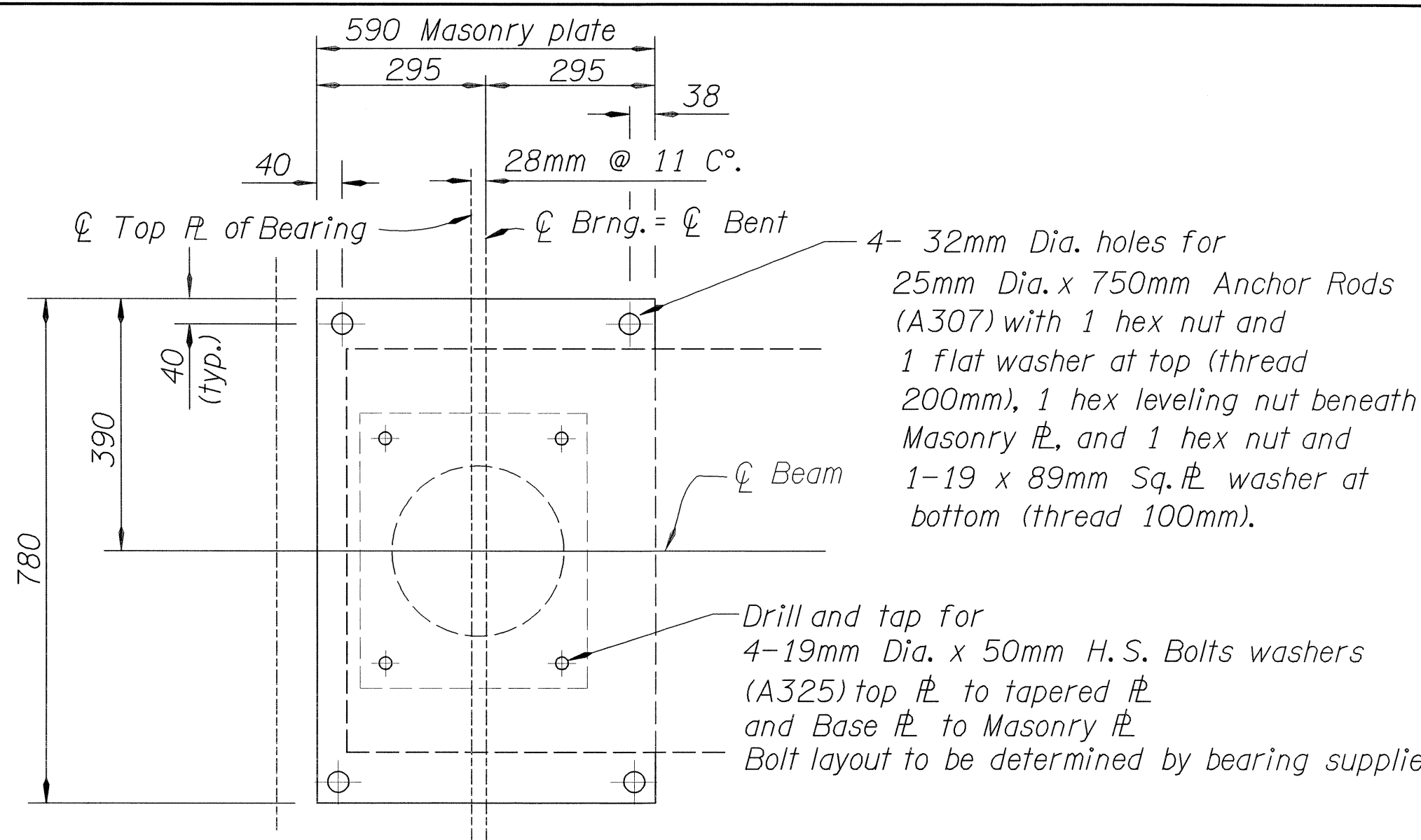
PARTIAL PLAN ~ BEAM-C
Scale 1:25



TYPICAL SECTION ~ BEAM-C
Scale 1:15

NOTE: All dimensions are in millimeters (mm) except as noted.

▲	DATE	REVISION	BY	DRAFTED: Philip G. Amaya	DESIGNER  EXPIRES: 6-30-04	BRIDGE NO. 19592	JACKSON SCHOOL ROAD OVER HWY 47 BEAM-C DETAILS	SHEET 14 OF 20
				CHECKED: Thiet Nguyen		 OREGON DEPARTMENT OF TRANSPORTATION BRIDGE ENGINEERING SECTION		DATE 19-Mar-2004
				REVIEWED: Hormoz Seradj		CALC. BOOK 5210-5211		



TYPICAL CONCRETE PEDESTAL
No Scale

EXPANSION BEARING DESIGN NOTES:

Select a pot bearing, disk bearing, or spherical bearing from the Department's QPL.
Provide 9 multi-directional and two guided bearings each at Bents 1 and 3.
Locate guided bearings under the first interior girders as shown on dwg. 65081
Provide a minimum rotational capacity of 0.015 Radians.
Secure the upper and lower components of the bearing to tapered plates and masonry.
Paint all exposed metal surfaces, except contact surfaces, per 00582.38.

Provide additional top and btm. plates as shown so that bearings can be removed with minimal jacking.
Ensure bearings are level in their final position.
Design movements for a 11° C. mean temperature are:
@ Max. movement @ 17°C Temperature Rise : 8mm
@ Max. movement @ 22°C Temperature Fall : -10mm
Creep and shrinkage : 28mm
Elastic shortening : 6mm
Change in Brg./10° C = 4.6mm
Provide a stainless steel sliding surface with the design movement plus 50 mm in each longitudinal direction.

NOTE: All dimensions are in millimeters (mm) except as noted.

	Bent 1	Bent 3
Dead Load	650 kN	641 kN
Live Load	283 kN	282 kN
Total Vertical Capacity Req'd.	933 kN	923 kN
Minimum Horizontal Capacity for Guided Bearings	275 kN	275 kN

DATE	REVISION	BY	DESIGNER	BRIDGE NO.	SHEET
			Philip G. Amaya	19592	20
			Thiet Nguyen	DATE	OF
			Hormoz Seradj	19-Mar-2004	20
				CALC. BOOK	DRAWING NO.
				5210-5211	65095
				JACKSON SCHOOL ROAD OVER HWY 47	
				BEARING DETAILS	